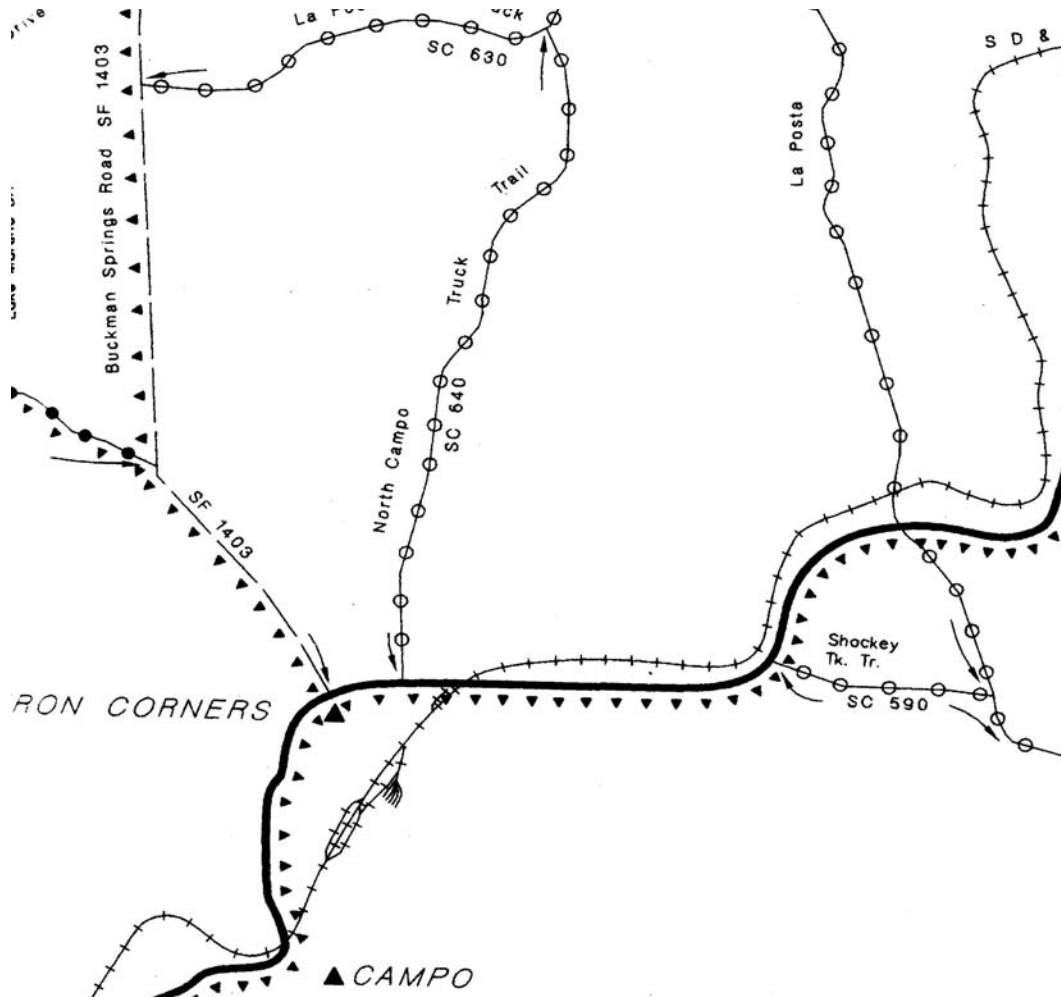


APPENDIX A

COUNTY OF SAN DIEGO CIRCULATION ELEMENT MAP AROUND STUDY AREA

San Diego County Circulation Element



LEGEND			
	FREEWAYS		THREE LANE COLLECTOR
	FREEWAYS (Proposed)		RURAL COLLECTORS
	EXPRESSWAYS		RURAL LIGHT COLLECTORS
	PRIME ARTERIALS		LIGHT COLLECTORS
	MAJOR ROADS		RURAL MOUNTAIN ROADS
	RECREATIONAL PARKWAYS		BICYCLE NETWORK SYSTEM
	COLLECTOR ROADS		INCORPORATED CITIES

APPENDIX B

COUNT DATA

LOS Engineering, Inc.

6342 Ferris Square, San Diego, CA 92121

Counted By: Emp. #06

Start Date: 03/10/2005

Location: SR-94 & Forest Gate Road

File Name: 347-02-1

	SR-94 Southbound				Forest Gate Road Westbound				SR-94 Northbound				Forest Gate Road Eastbound				
Start Time	Left	Thru	Right	Ped	Left	Thru	Right	Ped	Left	Thru	Right	Ped	Left	Thru	Right	Ped	Interval Total
7:00	14	4	0	0	1	0	3	0	0	11	4	0	1	0	0	0	38
7:15	7	8	1	0	1	1	13	0	1	13	2	0	0	0	1	0	48
7:30	13	12	0	0	2	0	8	0	0	14	3	0	1	0	0	0	53
7:45	13	9	1	0	0	0	8	0	1	3	5	0	0	0	0	0	40
Total	47	33	2	0	4	1	32	0	2	41	14	0	2	0	1	0	179
8:00	4	6	2	0	0	1	4	0	0	12	5	0	1	1	1	0	37
8:15	9	7	0	0	3	0	15	0	0	8	3	0	0	0	0	0	45
8:30	4	7	0	0	5	1	7	0	2	5	2	0	1	1	0	0	35
8:45	7	8	0	0	2	1	8	0	0	6	2	0	1	0	1	0	36
Total	24	28	2	0	10	3	34	0	2	31	12	0	3	2	2	0	153
Grand Total	71	61	4	0	14	4	66	0	4	72	26	0	5	2	3	0	332
Approach%	52.2	44.9	2.9	-	16.7	4.8	78.6	-	3.9	70.6	25.5	-	50.0	20.0	30.0	-	
Total%	21.4	18.4	1.2	-	4.2	1.2	19.9	-	1.2	21.7	7.8	-	1.5	0.6	0.9	-	

Peak hour analysis for the period 07:00 to 07:45

Volume	47	33	2	-	4	1	32	-	2	41	14	-	2	-	1	-	179
Approach%	57.3	40.2	2.4	-	10.8	2.7	86.5	-	3.5	71.9	24.6	-	66.7	-	33.3	-	
Total%	26.3	18.4	1.1	-	2.2	0.6	17.9	-	1.1	22.9	7.8	-	1.1	-	0.6	-	
PHF				0.82				0.62				0.84				0.75	

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LOS Engineering, Inc.

6342 Ferris Square, San Diego, CA 92121

Counted By: Emp. #06

Location: SR-94 & Forest Gate Road

Start Date: 03/10/2005

File Name: 347-02-2

	SR-94 Southbound				Forest Gate Road Westbound				SR-94 Northbound				Forest Gate Road Eastbound				
Start Time	Left	Thru	Right	Ped	Left	Thru	Right	Ped	Left	Thru	Right	Ped	Left	Thru	Right	Ped	Interval Total
16:00	11	13	0	0	7	1	10	0	2	10	0	0	0	2	0	0	56
16:15	12	8	2	4	4	3	9	0	1	6	0	0	3	2	0	0	54
16:30	11	11	3	0	2	0	15	0	1	3	0	0	4	2	0	0	52
16:45	5	6	2	0	3	6	7	0	0	8	1	2	2	1	0	2	45
Total	39	38	7	4	16	10	41	0	4	27	1	2	9	7	0	2	207
17:00	7	12	1	0	4	2	12	0	1	12	0	0	2	3	1	0	57
17:15	10	7	1	0	4	1	5	0	1	11	2	0	1	1	0	0	44
17:30	9	13	1	0	1	3	5	0	1	4	3	0	0	1	0	0	41
17:45	10	11	1	0	0	0	4	0	0	9	0	0	2	2	0	0	39
Total	36	43	4	0	9	6	26	0	3	36	5	0	5	7	1	0	181
Grand Total	75	81	11	4	25	16	67	0	7	63	6	2	14	14	1	2	388
Approach%	43.9	47.4	6.4	2.3	23.1	14.8	62.0	-	9.0	80.8	7.7	2.6	45.2	45.2	3.2	6.5	
Total%	19.3	20.9	2.8	1.0	6.4	4.1	17.3	-	1.8	16.2	1.5	0.5	3.6	3.6	0.3	0.5	

Peak hour analysis for the period 16:15 to 17:00

Volume	35	37	8	4	13	11	43	-	3	29	1	2	11	8	1	2	208
Approach%	41.7	44.0	9.5	4.8	19.4	16.4	64.2	-	8.6	82.9	2.9	5.7	50.0	36.4	4.5	9.1	
Total%	16.8	17.8	3.8	1.9	6.3	5.3	20.7	-	1.4	13.9	0.5	1.0	5.3	3.8	0.5	1.0	
PHF				0.81				0.93				0.67				0.92	

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Vehicle Counts

VehicleCount-347.2.1-NB

Datasets:

Site: [2.1] Buckman Springs Rd, btwn I-8 and SR-94
Direction: 5 - South bound A>B, North bound B>A., **Lane:** 0
Survey Duration: 18:07 Wednesday, March 09, 2005 => 1:50 Saturday, March 12, 2005
File: UM2.1312.EC0 (Plus)
Identifier: N116JRX9 MC56-L4 [MC55] (c)Microcom 19Sep03
Algorithm: **Modified** - Factory default
Data type: Axle sensors - Paired (Class, Speed, Count)

Profile:

Filter time: 0:00 Thursday, March 10, 2005 => 0:00 Friday, March 11, 2005
Included classes: 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13
Speed range: 5 - 100 mph.
Direction: North (bound)
Separation: All - (Headway)
Name: Factory default profile
Scheme: Vehicle classification (Scheme F99)
Units: Non metric (ft, mi, ft/s, mph, lb, ton)
In profile: Vehicles = 1305 / 3625 (36.00%)

* Thursday, March 10, 2005 - Total=1305, 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	
3	2	4	10	14	50	82	95	103	67	56	62	87	72	92	144	101	85	72	36	27	23	13	5	-
0	0	0	2	2	11	12	35	18	24	15	12	21	13	18	48	21	27	28	7	8	16	3	1	-
0	0	2	5	2	12	17	24	26	18	13	26	24	12	17	35	29	32	20	15	7	2	5	3	-
2	1	0	1	4	13	26	21	29	13	14	16	24	24	22	34	23	11	13	9	6	1	2	0	-
1	1	2	2	6	14	27	15	30	12	14	8	18	23	35	27	28	15	11	5	6	4	3	1	-

AM Peak 0630 - 0730 (112), AM PHF=0.80

Vehicle Counts

VehicleCount-347.2.1-SB

Datasets:

Site: [2.1] Buckman Springs Rd, btwn I-8 and SR-94
Direction: 5 - South bound A>B, North bound B>A., Lane: 0
Survey Duration: 18:07 Wednesday, March 09, 2005 => 1:50 Saturday, March 12, 2005
File: UM2.1312.EC0 (Plus)
Identifier: N116JRX9 MC56-L4 [MC55] (c)Microcom 19Sep03
Algorithm: Modified - Factory default
Data type: Axle sensors - Paired (Class, Speed, Count)

Profile:

Filter time: 0:00 Thursday, March 10, 2005 => 0:00 Friday, March 11, 2005
Included classes: 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13
Speed range: 5 - 100 mph.
Direction: South (bound)
Separation: All - (Headway)
Name: Factory default profile
Scheme: Vehicle classification (Scheme F99)
Units: Non metric (ft, mi, ft/s, mph, lb, ton)
In profile: Vehicles = 1311 / 3625 (36.17%)

* Thursday, March 10, 2005 - Total=1311, 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	
2	2	9	3	9	29	96	106	88	77	58	73	63	59	83	113	108	121	66	49	25	33	21	18	
0	0	3	0	1	5	10	29	17	23	17	14	13	14	12	29	26	26	23	15	6	10	5	6	-
0	0	3	2	3	5	22	18	25	18	12	19	14	16	24	33	28	30	15	8	10	5	0	4	-
1	2	0	1	2	8	26	27	25	20	11	17	15	16	33	27	38	35	12	16	4	11	10	2	-
1	0	3	0	3	11	38	32	21	16	18	23	21	13	14	24	16	30	16	10	5	7	6	6	-

AM Peak 0615 - 0715 (115), AM PHF=0.76

Vehicle Counts

VehicleCount-347.1.1-NB

Datasets:

Site: [1.1] Sheriden Rd, 50' s/o White Sage
Direction: 5 - South bound A>B, North bound B>A., Lane: 0
Survey Duration: 18:07 Wednesday, March 09, 2005 => 1:50 Saturday, March 12, 2005
File: UM1.1312.EC0 (Plus)
Identifier: N0269ZK4 MC56-L4 [MC55] (c)Microcom 19Sep03
Algorithm: Modified - Factory default
Data type: Axle sensors - Paired (Class, Speed, Count)

Profile:

Filter time: 0:00 Thursday, March 10, 2005 => 0:00 Friday, March 11, 2005
Included classes: 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13
Speed range: 5 - 100 mph.
Direction: North (bound)
Separation: All - (Headway)
Name: Factory default profile
Scheme: Vehicle classification (Scheme F99)
Units: Non metric (ft, mi, ft/s, mph, lb, ton)
In profile: Vehicles = 205 / 618 (33.17%)

* Thursday, March 10, 2005 - Total=205, 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	
4	0	1	1	3	3	11	16	9	8	10	11	14	16	15	19	15	14	9	5	4	2	2	13	-
0	0	1	0	1	1	0	6	2	4	2	7	0	4	4	4	3	4	2	1	1	1	2	0	-
0	0	0	0	0	1	2	3	4	0	2	3	4	8	5	5	2	4	4	4	0	0	0	4	-
4	0	0	1	1	0	4	5	2	2	4	1	6	3	4	5	5	1	1	0	3	0	0	5	-
0	0	0	0	1	1	5	2	1	2	2	0	4	1	2	5	5	5	2	0	0	1	0	4	-

AM Peak 0645 - 0745 (19), AM PHF=0.79

Vehicle Counts

VehicleCount-347.1.1-SB

Datasets:

Site: [1.1] Sheriden Rd, 50' s/o White Sage
Direction: 5 - South bound A>B, North bound B>A., Lane: 0
Survey Duration: 18:07 Wednesday, March 09, 2005 => 1:50 Saturday, March 12, 2005
File: UM1.1312.EC0 (Plus)
Identifier: N0269ZK4 MC56-L4 [MC55] (c)Microcom 19Sep03
Algorithm: Modified - Factory default
Data type: Axle sensors - Paired (Class, Speed, Count)

Profile:

Filter time: 0:00 Thursday, March 10, 2005 => 0:00 Friday, March 11, 2005
Included classes: 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13
Speed range: 5 - 100 mph.
Direction: South (bound)
Separation: All - (Headway)
Name: Factory default profile
Scheme: Vehicle classification (Scheme F99)
Units: Non metric (ft, mi, ft/s, mph, lb, ton)
In profile: Vehicles = 242 / 618 (39.16%)

* Thursday, March 10, 2005 - Total=242, 15 minute drops

0000	0100	0200	0300	0400	0500	0600	0700	0800	0900	1000	1100	1200	1300	1400	1500	1600	1700	1800	1900	2000	2100	2200	2300	
2	1	2	1	0	2	19	20	14	11	13	13	8	17	20	27	21	14	9	8	2	2	5	11	-
0	1	2	0	0	0	1	1	3	4	2	3	3	2	4	4	7	1	1	1	0	0	0	0	-
2	0	0	0	0	0	4	6	4	1	4	3	4	5	4	4	6	4	5	3	0	2	1	0	-
0	0	0	1	0	1	4	6	5	5	3	4	1	4	4	8	5	4	3	2	2	0	2	5	-
0	0	0	0	0	1	10	7	2	1	4	3	0	6	8	11	3	5	0	2	0	0	2	6	-

AM Peak 0645 - 0745 (23), AM PHF=0.57

2004 VOLUMES

District	Route	Rte		PM	Prefix	Postmile	Description	Back Peak	Back Peak	Back	Ahead	Ahead	Ahead
		Suf	County					Hour	Month	AADT	Peak Hour	Peak Month	AADT
11	94	SD				8.27	LEMON GROVE,	11500	145000	144000	11800	149000	148000
11	94	SD				8.77	LEMON GROVE, WAITE DRIVE	11800	149000	148000	12100	143000	141000
11	94	SD				8.98	LEMON GROVE, LEMON GROVE	12100	143000	141000	12400	146000	144000
11	94	SD				10.01	JCT. RTE. 125 (MILEPOST	12400	146000	144000	7000	82000	81000
11	94	SD		R		11.08	SPRING VALLEY, BANCROFT	7000	82000	81000	6400	75000	74000
11	94	SD		R		11.8	CASA DE ORO, KENWOOD DRIVE	6400	75000	74000	5300	63000	62000
11	94	SD		R		12.75	SWEETWATER SPRINGS	5300	63000	62000	4900	59000	57000
11	94	SD		R		13.14	END FREEWAY						
11	94	SD		R		13.33	AVOCADO BOULEVARD	4900	59000	57000	4150	50000	48500
11	94	SD		R		13.54	MILEPOST EQUATION =13.59						
11	94	SD				14.33	JAMACHA ROAD	4150	50000	48500	5500	66000	64000
11	94	SD				14.86	JCT. RTE. 54 NORTH	5500	66000	64000	1900	22900	22300
11	94	SD				17.35	STEELE CANYON ROAD	1900	22900	22300	1650	19900	19400
11	94	SD		R		19.27	MILEPOST EQUATION =19.35						
11	94	SD				19.4	LYONS VALLEY ROAD	1650	19900	19400	1100	13100	12800
11	94	SD				24.55	HONEY SPRINGS ROAD	760	8600	8500	740	8400	8300
11	94	SD				28.3	DULZURA POST OFFICE	740	8400	8300	770	8800	8700
11	94	SD				38.97	JCT. RTE. 188 SOUTH, TECATE	590	6900	6600	200	2800	2500
11	94	SD		R		45.48	MILEPOST EQUATION =45.57						
11	94	SD		R		47.38	MILEPOST EQUATION =47.42						
11	94	SD		R		50.59	MILEPOST EQUATION =50.63						
11	94	SD				52.15	BUCKMAN SPRINGS ROAD	170	2350	2100	180	2450	2200
11	94	SD				64.23	WHITE STAR, JUNCTION OLD	110	920	900	210	1850	1800
11	94	SD				64.82	JEWEL VALLEY/RIBBONWOOD	210	1850	1800	110	920	900
11	94	SD				65.38	JCT. RTE. 8	110	920	900			

Speed Statistics

SpeedStat-347.2.1-NB

Site: 2.1.0NS
Description: Buckman Springs Rd, btwn I-8 and SR-94
Filter time: 0:00 Thursday, March 10, 2005 => 0:00 Friday, March 11, 2005
Scheme: Vehicle classification (Scheme F99)
Filter: Cls(1 2 3 4 5 6 7 8 9 10 11 12 13) Dir(N) Sp(5,100) Sep(>0)

Vehicles = 1305

Posted speed limit= 55 mph, Exceeding = 760 (58.24%), Mean Exceeding = 61.53 mph

Maximum = 93.9 mph, **Minimum =** 8.4 mph, **Mean =** 56.3 mph

85% Speed = 64.0 mph, **95% Speed =** 69.1 mph, **Median =** 56.4 mph

10 mph Pace = 52 - 62, **Number in Pace =** 677 (51.88%)

Variance = 68.72, **Standard Deviation =** 8.29 mph

Speed Bins

Speed	Bin	Below	Above	Energy	vMult	n * vMult
0 - 5	0 0.0%	0 0.0%	1305 100.0%	0.00	0.00	0.00
5 - 10	1 0.1%	1 0.1%	1304 99.9%	0.00	0.00	0.00
10 - 15	2 0.2%	3 0.2%	1302 99.8%	0.00	0.00	0.00
15 - 20	3 0.2%	6 0.5%	1299 99.5%	0.00	0.00	0.00
20 - 25	0 0.0%	6 0.5%	1299 99.5%	0.00	0.00	0.00
25 - 30	1 0.1%	7 0.5%	1298 99.5%	0.00	0.00	0.00
30 - 35	11 0.8%	18 1.4%	1287 98.6%	0.00	0.00	0.00
35 - 40	20 1.5%	38 2.9%	1267 97.1%	0.00	0.00	0.00
40 - 45	64 4.9%	102 7.8%	1203 92.2%	0.00	0.00	0.00
45 - 50	135 10.3%	237 18.2%	1068 81.8%	0.00	0.00	0.00
50 - 55	308 23.6%	545 41.8%	760 58.2%	0.00	0.00	0.00
55 - 60	346 26.5%	891 68.3%	414 31.7%	0.00	0.00	0.00
60 - 65	250 19.2%	1141 87.4%	164 12.6%	0.00	0.00	0.00
65 - 70	112 8.6%	1253 96.0%	52 4.0%	0.00	0.00	0.00
70 - 75	41 3.1%	1294 99.2%	11 0.8%	0.00	0.00	0.00
75 - 80	8 0.6%	1302 99.8%	3 0.2%	0.00	0.00	0.00
80 - 85	1 0.1%	1303 99.8%	2 0.2%	0.00	0.00	0.00
85 - 90	1 0.1%	1304 99.9%	1 0.1%	0.00	0.00	0.00
90 - 95	1 0.1%	1305 100.0%	0 0.0%	0.00	0.00	0.00
95 - 100	0 0.0%	1305 100.0%	0 0.0%	0.00	0.00	0.00

Total Speed Rating = 0.00

Total Moving Energy (Estimated) = 0.00

Speed limit fields

Limit	Below	Above
0 55 (PSL)	545 41.8%	760 58.2%

Speed Statistics

SpeedStat-347.2.1-SB

Site: 2.1.0NS
Description: Buckman Springs Rd, btwn I-8 and SR-94
Filter time: 0:00 Thursday, March 10, 2005 => 0:00 Friday, March 11, 2005
Scheme: Vehicle classification (Scheme F99)
Filter: Cls(1 2 3 4 5 6 7 8 9 10 11 12 13) Dir(S) Sp(5,100) Sep(>0)

Vehicles = 1311

Posted speed limit= 55 mph, Exceeding = 892 (68.04%), Mean Exceeding = 62.61 mph

Maximum = 87.1 mph, **Minimum =** 14.7 mph, **Mean =** 58.5 mph

85% Speed = 66.2 mph, **95% Speed =** 71.1 mph, **Median =** 58.6 mph

10 mph Pace = 52 - 62, **Number in Pace =** 667 (50.88%)

Variance = 66.17, **Standard Deviation =** 8.13 mph

Speed Bins

Speed	Bin	Below	Above	Energy	vMult	n * vMult
0 - 5	0 0.0%	0 0.0%	1311 100.0%	0.00	0.00	0.00
5 - 10	0 0.0%	0 0.0%	1311 100.0%	0.00	0.00	0.00
10 - 15	1 0.1%	1 0.1%	1310 99.9%	0.00	0.00	0.00
15 - 20	2 0.2%	3 0.2%	1308 99.8%	0.00	0.00	0.00
20 - 25	1 0.1%	4 0.3%	1307 99.7%	0.00	0.00	0.00
25 - 30	0 0.0%	4 0.3%	1307 99.7%	0.00	0.00	0.00
30 - 35	4 0.3%	8 0.6%	1303 99.4%	0.00	0.00	0.00
35 - 40	14 1.1%	22 1.7%	1289 98.3%	0.00	0.00	0.00
40 - 45	34 2.6%	56 4.3%	1255 95.7%	0.00	0.00	0.00
45 - 50	108 8.2%	164 12.5%	1147 87.5%	0.00	0.00	0.00
50 - 55	255 19.5%	419 32.0%	892 68.0%	0.00	0.00	0.00
55 - 60	347 26.5%	766 58.4%	545 41.6%	0.00	0.00	0.00
60 - 65	293 22.3%	1059 80.8%	252 19.2%	0.00	0.00	0.00
65 - 70	164 12.5%	1223 93.3%	88 6.7%	0.00	0.00	0.00
70 - 75	63 4.8%	1286 98.1%	25 1.9%	0.00	0.00	0.00
75 - 80	13 1.0%	1299 99.1%	12 0.9%	0.00	0.00	0.00
80 - 85	10 0.8%	1309 99.8%	2 0.2%	0.00	0.00	0.00
85 - 90	2 0.2%	1311 100.0%	0 0.0%	0.00	0.00	0.00
90 - 95	0 0.0%	1311 100.0%	0 0.0%	0.00	0.00	0.00
95 - 100	0 0.0%	1311 100.0%	0 0.0%	0.00	0.00	0.00

Total Speed Rating = 0.00

Total Moving Energy (Estimated) = 0.00

Speed limit fields

Limit	Below	Above
0 55 (PSL)	419 32.0%	892 68.0%

Speed Statistics

SpeedStat-372.5.2-EB

Site: 3720101.0WE
Description: Eastbound Speed 1.2 Miles East of La Posta Rd on SR-94
Filter time: 0:00 Wednesday, June 01, 2005 => 0:00 Thursday, June 02, 2005
Scheme: Vehicle classification (Scheme F99)
Filter: Cls(1 2 3 4 5 6 7 8 9 10 11 12 13) Dir(W) Sp(5,100) Sep(>0)

Vehicles = 26

Posted speed limit= 55 mph, Exceeding = 1 (3.85%), Mean Exceeding = 55.90 mph

Maximum = 55.9 mph, **Minimum =** 24.0 mph, **Mean =** 42.8 mph

85% Speed = 50.6 mph, **95% Speed =** 53.9 mph, **Median =** 43.6 mph

10 mph Pace = 42 - 52, **Number in Pace =** 15 (57.69%)

Variance = 64.88, **Standard Deviation =** 8.05 mph

Speed Bins

Speed	Bin	Below	Above	Energy	vMult	n * vMult
0 - 5	0 0.0%	0 0.0%	26 100.0%	0.00	0.00	0.00
5 - 10	0 0.0%	0 0.0%	26 100.0%	0.00	0.00	0.00
10 - 15	0 0.0%	0 0.0%	26 100.0%	0.00	0.00	0.00
15 - 20	0 0.0%	0 0.0%	26 100.0%	0.00	0.00	0.00
20 - 25	1 3.8%	1 3.8%	25 96.2%	0.00	0.00	0.00
25 - 30	1 3.8%	2 7.7%	24 92.3%	0.00	0.00	0.00
30 - 35	2 7.7%	4 15.4%	22 84.6%	0.00	0.00	0.00
35 - 40	4 15.4%	8 30.8%	18 69.2%	0.00	0.00	0.00
40 - 45	8 30.8%	16 61.5%	10 38.5%	0.00	0.00	0.00
45 - 50	5 19.2%	21 80.8%	5 19.2%	0.00	0.00	0.00
50 - 55	4 15.4%	25 96.2%	1 3.8%	0.00	0.00	0.00
55 - 60	1 3.8%	26 100.0%	0 0.0%	0.00	0.00	0.00
60 - 65	0 0.0%	26 100.0%	0 0.0%	0.00	0.00	0.00
65 - 70	0 0.0%	26 100.0%	0 0.0%	0.00	0.00	0.00
70 - 75	0 0.0%	26 100.0%	0 0.0%	0.00	0.00	0.00
75 - 80	0 0.0%	26 100.0%	0 0.0%	0.00	0.00	0.00
80 - 85	0 0.0%	26 100.0%	0 0.0%	0.00	0.00	0.00
85 - 90	0 0.0%	26 100.0%	0 0.0%	0.00	0.00	0.00
90 - 95	0 0.0%	26 100.0%	0 0.0%	0.00	0.00	0.00
95 - 100	0 0.0%	26 100.0%	0 0.0%	0.00	0.00	0.00

Total Speed Rating = 0.00

Total Moving Energy (Estimated) = 0.00

Speed limit fields

Limit	Below	Above
0 55 (PSL)	25 96.2%	1 3.8%

Speed Statistics

SpeedStat-372.5.1-WB

Site: 3720102.0EW
Description: Westbound speed Survey 1.2 miles E/O La Posta Rd on SR-94
Filter time: 0:00 Wednesday, June 01, 2005 => 0:00 Thursday, June 02, 2005
Scheme: Vehicle classification (Scheme F99)
Filter: Cls(1 2 3 4 5 6 7 8 9 10 11 12 13) Dir(W) Sp(5,100) Sep(>0)

Vehicles = 426

Posted speed limit= 55 mph, Exceeding = 11 (2.58%), Mean Exceeding = 58.63 mph

Maximum = 72.7 mph, **Minimum =** 16.0 mph, **Mean =** 41.1 mph

85% Speed = 47.4 mph, **95% Speed =** 52.1 mph, **Median =** 41.2 mph

10 mph Pace = 37 - 47, **Number in Pace =** 252 (59.15%)

Variance = 55.29, **Standard Deviation =** 7.44 mph

Speed Bins

Speed	Bin	Below	Above	Energy	vMult	n * vMult
0 - 5	0 0.0%	0 0.0%	426 100.0%	0.00	0.00	0.00
5 - 10	0 0.0%	0 0.0%	426 100.0%	0.00	0.00	0.00
10 - 15	0 0.0%	0 0.0%	426 100.0%	0.00	0.00	0.00
15 - 20	2 0.5%	2 0.5%	424 99.5%	0.00	0.00	0.00
20 - 25	15 3.5%	17 4.0%	409 96.0%	0.00	0.00	0.00
25 - 30	20 4.7%	37 8.7%	389 91.3%	0.00	0.00	0.00
30 - 35	34 8.0%	71 16.7%	355 83.3%	0.00	0.00	0.00
35 - 40	99 23.2%	170 39.9%	256 60.1%	0.00	0.00	0.00
40 - 45	136 31.9%	306 71.8%	120 28.2%	0.00	0.00	0.00
45 - 50	80 18.8%	386 90.6%	40 9.4%	0.00	0.00	0.00
50 - 55	29 6.8%	415 97.4%	11 2.6%	0.00	0.00	0.00
55 - 60	9 2.1%	424 99.5%	2 0.5%	0.00	0.00	0.00
60 - 65	1 0.2%	425 99.8%	1 0.2%	0.00	0.00	0.00
65 - 70	0 0.0%	425 99.8%	1 0.2%	0.00	0.00	0.00
70 - 75	1 0.2%	426 100.0%	0 0.0%	0.00	0.00	0.00
75 - 80	0 0.0%	426 100.0%	0 0.0%	0.00	0.00	0.00
80 - 85	0 0.0%	426 100.0%	0 0.0%	0.00	0.00	0.00
85 - 90	0 0.0%	426 100.0%	0 0.0%	0.00	0.00	0.00
90 - 95	0 0.0%	426 100.0%	0 0.0%	0.00	0.00	0.00
95 - 100	0 0.0%	426 100.0%	0 0.0%	0.00	0.00	0.00

Total Speed Rating = 0.00

Total Moving Energy (Estimated) = 0.00

Speed limit fields

Limit	Below	Above
0 55 (PSL)	415 97.4%	11 2.6%

Speed Statistics

SpeedStat-411.1.1-NB

Site: 41101.0SN
Description: Sheridan Rd Btwn Custer Rd & Dodd Rd
Filter time: 0:00 Thursday, June 30, 2005 => 0:00 Friday, July 01, 2005
Scheme: Vehicle classification (Scheme F99)
Filter: Cls(1 2 3 4 5 6 7 8 9 10 11 12 13) Dir(N) Sp(5,100) Sep(>0)

Vehicles = 197

Posted speed limit= 35 mph, **Exceeding =** 97 (49.24%), **Mean Exceeding =** 40.18 mph

Maximum = 48.7 mph, **Minimum =** 11.0 mph, **Mean =** 34.8 mph

85% Speed = 41.8 mph, **95% Speed =** 44.3 mph, **Median =** 34.7 mph

10 mph Pace = 30 - 40, **Number in Pace =** 112 (56.85%)

Variance = 44.77, **Standard Deviation =** 6.69 mph

Speed Bins

Speed	Bin	Below	Above	Energy	vMult	n * vMult
0 - 5	0 0.0%	0 0.0%	197 100.0%	0.00	0.00	0.00
5 - 10	0 0.0%	0 0.0%	197 100.0%	0.00	0.00	0.00
10 - 15	2 1.0%	2 1.0%	195 99.0%	0.00	0.00	0.00
15 - 20	2 1.0%	4 2.0%	193 98.0%	0.00	0.00	0.00
20 - 25	12 6.1%	16 8.1%	181 91.9%	0.00	0.00	0.00
25 - 30	23 11.7%	39 19.8%	158 80.2%	0.00	0.00	0.00
30 - 35	61 31.0%	100 50.8%	97 49.2%	0.00	0.00	0.00
35 - 40	50 25.4%	150 76.1%	47 23.9%	0.00	0.00	0.00
40 - 45	38 19.3%	188 95.4%	9 4.6%	0.00	0.00	0.00
45 - 50	9 4.6%	197 100.0%	0 0.0%	0.00	0.00	0.00
50 - 55	0 0.0%	197 100.0%	0 0.0%	0.00	0.00	0.00
55 - 60	0 0.0%	197 100.0%	0 0.0%	0.00	0.00	0.00
60 - 65	0 0.0%	197 100.0%	0 0.0%	0.00	0.00	0.00
65 - 70	0 0.0%	197 100.0%	0 0.0%	0.00	0.00	0.00
70 - 75	0 0.0%	197 100.0%	0 0.0%	0.00	0.00	0.00
75 - 80	0 0.0%	197 100.0%	0 0.0%	0.00	0.00	0.00
80 - 85	0 0.0%	197 100.0%	0 0.0%	0.00	0.00	0.00
85 - 90	0 0.0%	197 100.0%	0 0.0%	0.00	0.00	0.00
90 - 95	0 0.0%	197 100.0%	0 0.0%	0.00	0.00	0.00
95 - 100	0 0.0%	197 100.0%	0 0.0%	0.00	0.00	0.00

Total Speed Rating = 0.00

Total Moving Energy (Estimated) = 0.00

Speed limit fields

Limit	Below	Above
0 35 (PSL)	100 50.8%	97 49.2%

Speed Statistics

SpeedStat-411.1.1-SB

Site: 41101.0SN
Description: Sheridan Rd Btwn Custer Rd & Dodd Rd
Filter time: 0:00 Thursday, June 30, 2005 => 0:00 Friday, July 01, 2005
Scheme: Vehicle classification (Scheme F99)
Filter: Cls(1 2 3 4 5 6 7 8 9 10 11 12 13) Dir(S) Sp(5,100) Sep(>0)

Vehicles = 243

Posted speed limit= 35 mph, Exceeding = 100 (41.15%), Mean Exceeding = 38.87 mph

Maximum = 49.2 mph, **Minimum =** 14.1 mph, **Mean =** 33.2 mph

85% Speed = 39.6 mph, **95% Speed =** 41.6 mph, **Median =** 33.3 mph

10 mph Pace = 31 - 41, **Number in Pace =** 145 (59.67%)

Variance = 38.03, **Standard Deviation =** 6.17 mph

Speed Bins

Speed	Bin	Below	Above	Energy	vMult	n * vMult
0 - 5	0 0.0%	0 0.0%	243 100.0%	0.00	0.00	0.00
5 - 10	0 0.0%	0 0.0%	243 100.0%	0.00	0.00	0.00
10 - 15	2 0.8%	2 0.8%	241 99.2%	0.00	0.00	0.00
15 - 20	6 2.5%	8 3.3%	235 96.7%	0.00	0.00	0.00
20 - 25	15 6.2%	23 9.5%	220 90.5%	0.00	0.00	0.00
25 - 30	45 18.5%	68 28.0%	175 72.0%	0.00	0.00	0.00
30 - 35	75 30.9%	143 58.8%	100 41.2%	0.00	0.00	0.00
35 - 40	66 27.2%	209 86.0%	34 14.0%	0.00	0.00	0.00
40 - 45	33 13.6%	242 99.6%	1 0.4%	0.00	0.00	0.00
45 - 50	1 0.4%	243 100.0%	0 0.0%	0.00	0.00	0.00
50 - 55	0 0.0%	243 100.0%	0 0.0%	0.00	0.00	0.00
55 - 60	0 0.0%	243 100.0%	0 0.0%	0.00	0.00	0.00
60 - 65	0 0.0%	243 100.0%	0 0.0%	0.00	0.00	0.00
65 - 70	0 0.0%	243 100.0%	0 0.0%	0.00	0.00	0.00
70 - 75	0 0.0%	243 100.0%	0 0.0%	0.00	0.00	0.00
75 - 80	0 0.0%	243 100.0%	0 0.0%	0.00	0.00	0.00
80 - 85	0 0.0%	243 100.0%	0 0.0%	0.00	0.00	0.00
85 - 90	0 0.0%	243 100.0%	0 0.0%	0.00	0.00	0.00
90 - 95	0 0.0%	243 100.0%	0 0.0%	0.00	0.00	0.00
95 - 100	0 0.0%	243 100.0%	0 0.0%	0.00	0.00	0.00

Total Speed Rating = 0.00

















Total Moving Energy (Estimated) = 0.00










Speed limit fields

















Limit	Below	Above
0 35 (PSL)	143 58.8%	100 41.2%











APPENDIX C

EXISTING INTERSECTION LEVEL OF SERVICE CALCUALTIONS

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	2	0	1	4	1	32	2	41	14	47	33	2
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	2	0	1	4	1	34	2	43	15	49	35	2
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	224	197	36	191	191	51	37			58		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	224	197	36	191	191	51	37			58		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	99	100	97	100			97		
cM capacity (veh/h)	689	675	1037	749	681	1018	1574			1546		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	3	39	60	86								
Volume Left	2	4	2	49								
Volume Right	1	34	15	2								
cSH	776	967	1574	1546								
Volume to Capacity	0.00	0.04	0.00	0.03								
Queue Length 95th (ft)	0	3	0	2								
Control Delay (s)	9.7	8.9	0.3	4.3								
Lane LOS	A	A	A	A								
Approach Delay (s)	9.7	8.9	0.3	4.3								
Approach LOS	A	A										
Intersection Summary												
Average Delay			4.1									
Intersection Capacity Utilization			21.1%		ICU Level of Service					A		
Analysis Period (min)			15									

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Volume (veh/h)	50	22	16	42	10	6
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	53	23	17	44	11	6
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type					None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			76		142	64
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			76		142	64
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		99	99
cM capacity (veh/h)			1523		841	1000
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	76	61	17			
Volume Left	0	17	11			
Volume Right	23	0	6			
cSH	1700	1523	895			
Volume to Capacity	0.04	0.01	0.02			
Queue Length 95th (ft)	0	1	1			
Control Delay (s)	0.0	2.1	9.1			
Lane LOS		A	A			
Approach Delay (s)	0.0	2.1	9.1			
Approach LOS			A			
Intersection Summary						
Average Delay			1.8			
Intersection Capacity Utilization		19.8%		ICU Level of Service		A
Analysis Period (min)		15				

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	11	8	1	13	11	43	3	29	1	35	37	8
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	12	8	1	14	12	45	3	31	1	37	39	8
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	205	155	43	159	158	31	47			32		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	205	155	43	159	158	31	47			32		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	98	99	100	98	98	96	100			98		
cM capacity (veh/h)	697	718	1027	783	715	1043	1560			1581		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	21	71	35	84								
Volume Left	12	14	3	37								
Volume Right	1	45	1	8								
cSH	717	915	1560	1581								
Volume to Capacity	0.03	0.08	0.00	0.02								
Queue Length 95th (ft)	2	6	0	2								
Control Delay (s)	10.2	9.3	0.7	3.3								
Lane LOS	B	A	A	A								
Approach Delay (s)	10.2	9.3	0.7	3.3								
Approach LOS	B	A										
Intersection Summary												
Average Delay			5.6									
Intersection Capacity Utilization			21.7%		ICU Level of Service					A		
Analysis Period (min)			15									

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Volume (veh/h)	63	8	9	44	21	12
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	66	8	9	46	22	13
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type					None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			75		136	71
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			75		136	71
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		97	99
cM capacity (veh/h)			1525		852	992
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	75	56	35			
Volume Left	0	9	22			
Volume Right	8	0	13			
cSH	1700	1525	898			
Volume to Capacity	0.04	0.01	0.04			
Queue Length 95th (ft)	0	0	3			
Control Delay (s)	0.0	1.3	9.2			
Lane LOS		A	A			
Approach Delay (s)	0.0	1.3	9.2			
Approach LOS			A			
Intersection Summary						
Average Delay		2.4				
Intersection Capacity Utilization		19.5%		ICU Level of Service		A
Analysis Period (min)		15				

APPENDIX D

CALTRANS' K AND D FACTORS

DI	RTE	CO	PRE	PM	CS	LEG	YR	Dir	AM PEAK					Dir	PM PEAK					HR	DAY	MNTH	
									1 WAY	%	%	%	1 WAY		%	%	%						
									PHV	K	D	KD		PHV	K	D	KD						
08	091	RIV		11.99	812	X	01	W	8373	8	66.42	5.31	11	WED	AUG	W	6276	7.53	52.89	3.98	17	FRI	DEC
08	091	RIV		21.66	985	B	01	E	5939	7.64	52.05	3.98	9	WED	DEC	E	5560	6.92	53.8	3.72	14	FRI	MAY
04	092	SM		5.191	57	A	03	W	1550	10.38	58.6	6.08	12	SUN	NOV	W	1527	9.32	64.27	5.99	17	TUE	NOV
04	092	SM	T	6.52	196	A	03	E	1254	7.45	74.91	5.58	7	WED	MAY	E	1335	9.36	63.48	5.94	17	SUN	MAY
04	092	SM	R	7.303	197	A	03	E	4653	11.26	52.67	5.93	8	WED	NOV	W	4514	11.23	51.26	5.75	17	THU	MAY
04	092	SM	R	11.21	198	B	03	E	4782	9.77	51.66	5.05	8	WED	MAY	W	4769	8.99	55.98	5.03	17	MON	MAY
04	092	ALA		8.219	32	B	03	W	2045	8.55	59.92	5.13	7	WED	NOV	E	2166	9.31	58.3	5.43	17	THU	AUG
11	094	SD		1.418	936	A	03	W	6948	8.09	81.14	6.56	7	TUE	JUL	E	7385	9.49	73.53	6.97	16	MON	FEB
11	094	SD		5.789	989	B	03	W	8696	7.49	67.55	5.06	7	WED	FEB	E	9417	8.44	64.89	5.48	17	FRI	FEB
11	094	SD		8.983	990	B	03	W	6943	7.04	69.64	4.91	6	MON	NOV	E	7961	8.69	64.71	5.63	16	TUE	APR
11	094	SD	R	13.33	959	B	03	W	2736	7.78	67.39	5.24	7	TUE	DEC	E	2883	9.16	60.34	5.52	17	WED	FEB
11	094	SD		24.55	975	B	03	W	542	8.6	75.91	6.53	6	WED	NOV	E	550	8.89	74.53	6.62	17	FRI	OCT
11	094	SD		38.97	856	B	03	W	640	11.02	90.14	9.93	6	TUE	MAY	E	563	11.41	76.6	8.74	17	FRI	MAY
11	094	SD		38.97	857	A	03	W	135	8.4	65.85	5.53	9	MON	MAY	E	167	10.16	67.34	6.84	14	FRI	MAY
11	094	SD		64.23	858	B	03	E	56	10.59	61.54	6.52	9	SAT	JUN	W	52	11.64	52	6.05	14	FRI	JUN
08	095	RIV		0	874	A	02	S	154	9.36	51.33	4.81	12	FRI	MAR	N	189	9.02	65.4	5.9	16	FRI	JUN
08	095	RIV		3.52	912	B	02	N	162	9.22	59.78	5.51	10	MON	MAR	S	190	9.29	69.6	6.47	15	SUN	JUN
08	095	RIV		3.52	918	A	02	N	136	8.75	63.26	5.53	10	SAT	MAR	S	167	9.56	71.06	6.79	18	SUN	JUN
08	095	RIV		10.54	903	A	02	N	112	10.88	55.72	6.06	11	MON	MAR	S	126	11.96	57.01	6.82	14	SUN	MAR
08	095	RIV		36.20	906	B	02	N	99	10.86	58.24	6.33	12	MON	MAR	N	100	11.95	53.48	6.39	13	FRI	MAR
08	095	SBD		9.684	848	A	02	N	194	11.03	60.63	6.69	12	MON	MAR	S	218	11.51	65.27	7.51	14	SUN	JUL
08	095	SBD		37.3	847	B	02	N	170	11.03	53.13	5.86	11	FRI	MAR	S	177	12.06	50.57	6.1	13	SUN	MAR
08	095	SBD		57.28	806	B	02	N	243	9.48	54.98	5.21	12	SUN	MAR	N	256	9.48	57.92	5.49	14	FRI	MAR
08	095	SBD	R	57.24	958	A	02	N	166	8.41	67.48	5.68	10	WED	MAR	N	201	12.24	56.15	6.87	13	SUN	MAR
01	096	HUM		3.59	108	O	02	W	105	7.97	69.08	5.5	12	SAT	AUG	W	118	11.43	54.13	6.18	16	FRI	NOV
02	096	SIS		41.10	135	A	02	W	106	9.45	51.71	4.88	10	MON	AUG	W	130	10.92	54.85	5.99	13	FRI	AUG
02	096	SIS		60.76	303	O	03	W	42	9.85	70	6.9	9	SAT	AUG	W	47	12.48	61.84	7.72	17	FRI	JUN
02	096	SIS		103.4	137	B	02	E	54	10.25	62.79	6.44	12	FRI	AUG	E	65	14.42	53.72	7.75	15	FRI	AUG
02	096	SIS		105.8	243	B	02	W	34	11.83	59.65	7.05	12	MON	MAY	W	40	14.52	57.14	8.3	18	FRI	AUG
02	097	SIS	L	0	268	A	02	N	516	8.79	50.59	4.45	12	FRI	AUG	N	604	9.88	52.71	5.21	17	FRI	AUG
02	097	SIS	L	.429	138	A	02	N	317	9.19	52.4	4.82	12	FRI	AUG	N	360	9.39	58.25	5.47	18	THU	AUG
02	097	SIS		20.19	183	B	03	S	195	11.05	56.03	6.19	10	SUN	AUG	S	203	11.91	54.13	6.45	15	SUN	AUG
02	097	SIS		36.15	311	B	03	S	219	11.93	60.33	7.2	12	SUN	JUL	N	210	13.54	50.97	6.9	14	SUN	AUG

APPENDIX E

EXISTING TWO-LANE HIGHWAY (HCM) CALCULATIONS

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 1/24/06
Analysis Time Period AM
Highway SR-94
From/To Tecate Rd to Forest Gate Rd
Jurisdiction Caltrans
Analysis Year Existing
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.8	mi	% Recreational vehicles	4	%
Terrain type	Rolling		% No-passing zones	100	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	138	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	0.71	
PCE for trucks, ET	2.5	
PCE for RVs, ER	1.1	
Heavy-vehicle adjustment factor,	0.824	
Two-way flow rate, (note-1) vp	268	pc/h
Highest directional split proportion (note-2)	180	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	3.8	mi/h
Average travel speed, ATS	48.9	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	0.77	
PCE for trucks, ET	1.8	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.899	
Two-way flow rate,(note-1) vp	226	pc/h
Highest directional split proportion (note-2)	151	
Base percent time-spent-following, BPTSF	18.0	%
Adj.for directional distribution and no-passing zones, fd/np	24.7	
Percent time-spent-following, PTSF	42.7	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.08	
Peak 15-min vehicle-miles of travel, VMT15	463	veh-mi
Peak-hour vehicle-miles of travel, VMT60	1628	veh-mi
Peak 15-min total travel time, TT15	9.5	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 1/24/06
Analysis Time Period PM
Highway SR-94
From/To Tecate Rd to Forest Gate Rd
Jurisdiction Caltrans
Analysis Year Existing
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.8	mi	% Recreational vehicles	4	%
Terrain type	Rolling		% No-passing zones	100	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	171	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	0.71	
PCE for trucks, ET	2.5	
PCE for RVs, ER	1.1	
Heavy-vehicle adjustment factor,	0.824	
Two-way flow rate, (note-1) vp	332	pc/h
Highest directional split proportion (note-2)	222	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	4.2	mi/h
Average travel speed, ATS	48.1	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	0.77	
PCE for trucks, ET	1.8	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.899	
Two-way flow rate, (note-1) vp	281	pc/h
Highest directional split proportion (note-2)	188	
Base percent time-spent-following, BPTSF	21.9	%
Adj.for directional distribution and no-passing zones, fd/np	24.1	
Percent time-spent-following, PTSF	46.0	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.10	
Peak 15-min vehicle-miles of travel, VMT15	573	veh-mi
Peak-hour vehicle-miles of travel, VMT60	2018	veh-mi
Peak 15-min total travel time, TT15	11.9	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 1/24/06
Analysis Time Period AM
Highway SR-94
From/To Forest Gate Rd to Buckman Spr
Jurisdiction Caltrans
Analysis Year Existing
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	1.4	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	138	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	172	pc/h
Highest directional split proportion (note-2)	115	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	1.6	mi/h
Average travel speed, ATS	51.8	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	159	pc/h
Highest directional split proportion (note-2)	107	
Base percent time-spent-following, BPTSF	13.0	%
Adj.for directional distribution and no-passing zones, fd/np	21.7	
Percent time-spent-following, PTSF	34.8	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.05	
Peak 15-min vehicle-miles of travel, VMT15	55	veh-mi
Peak-hour vehicle-miles of travel, VMT60	193	veh-mi
Peak 15-min total travel time, TT15	1.1	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 1/24/06
Analysis Time Period PM
Highway SR-94
From/To Forest Gate Rd to Buckman Spr
Jurisdiction Caltrans
Analysis Year Existing
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	1.4	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	171	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	213	pc/h
Highest directional split proportion (note-2)	143	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	2.0	mi/h
Average travel speed, ATS	51.2	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	197	pc/h
Highest directional split proportion (note-2)	132	
Base percent time-spent-following, BPTSF	15.9	%
Adj.for directional distribution and no-passing zones, fd/np	21.4	
Percent time-spent-following, PTSF	37.3	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.07	
Peak 15-min vehicle-miles of travel, VMT15	68	veh-mi
Peak-hour vehicle-miles of travel, VMT60	239	veh-mi
Peak 15-min total travel time, TT15	1.3	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 1/24/06
Analysis Time Period AM
Highway SR-94
From/To Buckman Springs to Sheridan Rd
Jurisdiction Caltrans
Analysis Year Existing
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	0.8	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	143	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	178	pc/h
Highest directional split proportion (note-2)	101	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	1.7	mi/h
Average travel speed, ATS	51.7	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	165	pc/h
Highest directional split proportion (note-2)	94	
Base percent time-spent-following, BPTSF	13.5	%
Adj.for directional distribution and no-passing zones, fd/np	19.6	
Percent time-spent-following, PTSF	33.1	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.06	
Peak 15-min vehicle-miles of travel, VMT15	33	veh-mi
Peak-hour vehicle-miles of travel, VMT60	114	veh-mi
Peak 15-min total travel time, TT15	0.6	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 1/24/06
Analysis Time Period PM
Highway SR-94
From/To Buckman Springs to Sheridan Rd
Jurisdiction Caltrans
Analysis Year Existing
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	0.8	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	133	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	166	pc/h
Highest directional split proportion (note-2)	95	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	1.6	mi/h
Average travel speed, ATS	51.9	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	153	pc/h
Highest directional split proportion (note-2)	87	
Base percent time-spent-following, BPTSF	12.6	%
Adj.for directional distribution and no-passing zones, fd/np	19.6	
Percent time-spent-following, PTSF	32.2	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.05	
Peak 15-min vehicle-miles of travel, VMT15	30	veh-mi
Peak-hour vehicle-miles of travel, VMT60	106	veh-mi
Peak 15-min total travel time, TT15	0.6	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 1/24/06
Analysis Time Period AM
Highway SR-94
From/To Sheridan Rd to White Star
Jurisdiction Caltrans
Analysis Year Existing
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.3	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	143	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	178	pc/h
Highest directional split proportion (note-2)	101	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	1.7	mi/h
Average travel speed, ATS	51.7	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	165	pc/h
Highest directional split proportion (note-2)	94	
Base percent time-spent-following, BPTSF	13.5	%
Adj.for directional distribution and no-passing zones, fd/np	19.6	
Percent time-spent-following, PTSF	33.1	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.06	
Peak 15-min vehicle-miles of travel, VMT15	459	veh-mi
Peak-hour vehicle-miles of travel, VMT60	1616	veh-mi
Peak 15-min total travel time, TT15	8.9	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 1/24/06
Analysis Time Period PM
Highway SR-94
From/To Sheridan Rd to White Star
Jurisdiction Caltrans
Analysis Year Existing
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.3	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	133	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	166	pc/h
Highest directional split proportion (note-2)	95	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	1.6	mi/h
Average travel speed, ATS	51.9	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	153	pc/h
Highest directional split proportion (note-2)	87	
Base percent time-spent-following, BPTSF	12.6	%
Adj.for directional distribution and no-passing zones, fd/np	19.6	
Percent time-spent-following, PTSF	32.2	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.05	
Peak 15-min vehicle-miles of travel, VMT15	427	veh-mi
Peak-hour vehicle-miles of travel, VMT60	1503	veh-mi
Peak 15-min total travel time, TT15	8.2	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

APPENDIX F

PROJECT INFORMATION

PROJECT DESCRIPTION

ST. ADELAIDE CATHOLIC CHURCH OPERATIONAL CHARACTERISTICS

This 5.13 acre site is to be used for the new location of the St. Adelaide Catholic Church. There will be three buildings, built over three phases. Water and sewer service are provided by the County of San Diego. Access is by Sheridan Road.

Total proposed building area is 22,804 square feet (including trash enclosure) plus 2,421 covered arcade ~~patio and lobby~~. Amenities include 136 parking spaces, a 10,000 square foot playground, ~~outdoor barbeque~~ and extensive landscaping.

Phase I: MULTI-PURPOSE ROOM ~~MEETING HALL~~/WORSHIP SPACE

The first phase will include the total infrastructure, parking (136 spaces), landscaping, water/sewer hookups and electrical power. It will also include the construction of the one story 5,656 square foot multi-purpose room ~~meeting hall~~, a 1,786 square foot covered arcade and a 180 square foot trash enclosure. The multi-purpose room ~~meeting hall~~ will be large enough to serve as a temporary worship space as well as to function as a ~~multi-purpose room~~ meeting hall. It will include a kitchenette, a small office, storage for tables and chairs, toilets, janitor and electrical service room. ~~An outdoor barbeque area will be provided. Barbecues will be provided in conjunction with the kitchenette. Outdoor dining is not contemplated.~~

There will be one Mass in the multi-use room on Saturday evening and one Mass in the multi-use room on Sunday morning. ~~Saturday evening will generate 40 cars trips and 65 people, between 4:30 p.m. and 7 p.m. The largest gathering will be 250 people Sunday morning between 9:30 a.m. noon which will generate a parking demand for approximately 86 cars. 230 people between 9:30 a.m. and noon. This is based upon a survey of vehicle occupancy indicating 2.9 persons per vehicle. Once a month Sept-June there will be an afternoon function on Sunday that will extend the hours until 3 p.m. and generate an additional 50 cars and 75 people after the Mass.~~

If the use of the church or the parking approaches capacity additional masses will be scheduled such that the parking and occupancy of the building stay within the designed capacity.

In addition there will be five weekdays when the multi-use room ~~it~~ will be used for Mass on our Holy Days of Obligation. ~~Christmas, Thanksgiving and First Friday Mass will also add to the use.~~ These additional masses will ~~primarily~~ be in the morning except for First Friday mass in the evening and only one or two masses, not four. First Friday Mass is in the evening. The building will be used from 4:30 pm-7:30 pm on that day. This will

involve around 100 people. These Masses will be transferred to the Sanctuary upon its construction.

Once a month Saturday events may be planned for fundraisers, gatherings and presentations. A fundraiser may generate ~~250-350~~ approximately 125-175 people attending. These events will generate a parking demand for approximately 88 vehicles based upon a vehicle occupancy of 2 per vehicle. ~~Education~~ Catechism classes will be held ~~on Sundays as well as~~ once a week during the school year in the evening, usually Wednesdays. In addition on the first Tuesday of each month there is an evening meeting from 6 p.m. until 8:30 p.m. ~~These are small gatherings with less than 100 people attending.~~

The meeting hall will be 25' 11" tall. The exterior will be off white or beige stucco. The roofing material will be muted red terracotta tile and the trellis will be stained brown.

Phase II: NEW SANCTUARY

The second phase, ~~the~~ construction and use of a new 4,809 square foot sanctuary and 635 square foot covered lobby, will be within five years of the completion of the first phase. This building will be the main worship site. It will have seating for approximately +/- 300 people and will include all of the necessary elements of a Catholic church: Altar, presider area, ambo, vesting room, lobby, sacristy, toilets, offices, counseling rooms and a sound/lighting room.

There will be one Mass Saturday evening between 4:30 p.m. and 7 p.m. and ~~one two~~ Mass on Sunday morning. ~~Saturday evening will generate 100 car trips and 175 people, between 4:30 p.m. and 7 p.m. Sunday morning between 9:30 and noon. will generate 150 cars and 270 people between 9:30 and noon. The same once-a-month function will generate an additional 75 cars and 150 people after the Mass. The occupancy of 300 people will generate a parking demand for approximately 103 vehicles, based upon a vehicle occupancy of 2.9 persons per vehicle.~~

~~Easter and Christmas will generate an additional 200 cars and 450 people between the two Masses. It may be necessary to celebrate an additional Mass on Sunday morning at 8:30, which will distribute the same number of people among three Masses instead of two.~~

There will be catechism classes on Wednesday evenings during the school year that will generate approximately 60 car trips and 80 people, including students and teachers. Once a month Saturday events may be planned for fundraisers, gatherings and presentations. A fundraiser may generate approximately 125-175 people. These events will generate a parking demand for approximately 88 vehicles based upon a vehicle occupancy of 2 per vehicle.

If the use of the church or the parking approaches capacity additional masses will be scheduled such that the parking and occupancy of the building stay within the designed capacity.

~~As the community grows and needs increase it may be necessary to assign a full time pastor to this community. This would increase the daily use of the facility with a daily mass either in the morning or the evening. Probably no more than 100 people would attend this Mass. It would also increase the daily use of the offices for counseling, record keeping, etc.; most likely no more than 10-15 trips a day. This number will increase as the population increases.~~

The height to the ridge of the roof will be 30' 10". The height to the top of the steeple will be 41'. The height to the top of the cross atop the steeple will be 47' 10". The construction materials and color will be the same as the multi purpose hall.

PHASE III: EDUCATION BUILDING

The third phase will be the building of a 12,159 square foot school facility. This will be built after the sanctuary, probably within ten years of the start of construction. This will be an elementary school, K-6th. It will also be one-story building similar to the sanctuary. The school structure will include administration, teacher offices, lounge, toilets and storage, classrooms, janitorial and electrical rooms.

There will be nine classrooms of 25 students each. A turf play yard of 10,000 square feet is included. The use will be the normal school year of September-June and will create approximately 275 270+/- people visits a day. Normal school hours will cover from 7 a.m. until 5 p.m. with parent/teacher meetings, assemblies, sporting events and some after school activities extending the hours into the evenings. The required yard space, security, first aid and support structures will all be a part of this phase. The school day will include a 40-minute outdoor recess period in the morning, a 40-minute recess period in the afternoon and a one-hour lunch period. The recess periods will be split such that only half of the students will be at recess at any one time.

The phase III parking demands will be divided between the weekend church services and weekday school requirements. The greatest phase III parking demands will occur during the weekend church services of up to 103 vehicles based upon a maximum attendance of 300 people as described under phase II. The weekday school parking requirements are estimated at approximately 30 parking spaces based upon the estimated number of teachers, staff, and volunteers.

The timing of the school construction will depend upon the demand for it and the finances available to pay for not only the construction costs but also the ongoing administration and upkeep.

The height of the school building will be 21' 10". The construction materials and colors will be the same as the other structures.

~~The Site~~ THE SITE

The subject property is a 5.13-acre site at the northeast corner of the intersection of Custer Road and Sheridan Road in the Campo Valley, just east of SR94 and the old San Diego & Arizona Eastern Railroad line.

The property shows obvious signs of relatively recent use shown by building foundations and debris. ~~The dominant vegetation is chaparral.~~ A County of San Diego sewer line transects the property.

The site has been graded in the past, as can be seen from the topo map generated for this development. A slope analysis has been provided. There are no Resource Protection Ordinance "steep slopes" on the site. The property slopes gently from the southeast, elevation 2639 msl, to the northwest elevation 2594' msl.

Mechanical Equipment

Mechanical equipment consisting of ground mounted heating/cooling units will be provided. The equipment will be screened and enclosed behind a masonry wall finished to match the building. Heights for the masonry enclosures are described in the acoustical study, ISE Report #05-127. A solid metal gate shall be installed with each bank of mechanical equipment. The proposed equipment consists of:

PHASE	EQUIPMENT
I. Multi-purpose Hall	One twenty-ton condensing unit and one 7.5-ton condensing unit or two ten-ton condensing units and one 7.5 ton condensing unit.
II. Sanctuary	Two 8-ton condensing units and one 6.5-ton condensing unit.
III. School	Nine four-ton condensing units and one 10-ton condensing unit.

PARKING SUMMARY

PROJECT PHASE	MAXIMUM ESTIMATED PARKING DEMAND BASED ON EXISTING VEHICLE OCCUPANCE (1)
<u>Phase I</u> Weekend Mass (Sat & Sun) Weekend Fundraiser (once a month) Weekday Evening Catechism Classes (once a week)	86 Parking Spaces 88 Parking Spaces (Max) 60 Parking Spaces
<u>Phase II</u> Weekend Mass (Sat & Sun) Weekend Fundraiser (once a month) Weekday Evening Catechism Classes (once a week)	103 Parking Spaces (Max) 88 Parking Spaces 60 Parking Spaces
<u>Phase III</u> Weekend Mass (Sat & Sun) Weekend Fundraiser (once a month) Weekday Evening Catechism Classes (once a week) Weekday School	103 Parking Spaces (Max) 88 Parking Spaces 60 Parking Spaces 30 Parking Spaces

A vehicle occupancy of 2.9 is from actual Church records that document the number of people in each family and the number of vehicles used to attend service.

CHURCH PARKING OCCUPANCY DATA

St. Adelaide, Campo			
FAMILY #	# IN FAMILY		# CAR TRIP
	Adults	Children	
#1	2		1
2	1		1
3	2	7	2
4	2	1	1
5	2		1
6	2		1
7	2	3	1
8	2		1
9	2		1
10	2	1	1
11	1		1
12	1		1
13	2	4	1
14	2	3	1
15	1	3	1
16	1		1

This is the first 40 of 85 registered families and is an educated guess of the number of people per car trip on Sunday for Mass in Campo

Total # of people is 118
Total car trips is 41
Occupancy = people/cars
= 118/41
= **2.9 people/car**

17	2	3	1
18	2		1
19	2		1
20	1		1
21	2		1
22	1	1	1
23	1		1
24	2		1
25	2	1	1
26	2		1
27	1	1	1
28	2	3	1
29	1		1
30	2		1
31	2	1	1
32	2	1	1
33	2		1
34	2		1
35	1	4	1
36	2	3	1
37	2	1	1
38	2	4	1
39	2		1
40	2	4	1

~~The Vicinity~~ THE VICINITY

The immediate vicinity includes scattered residential uses on large lots and a new subdivision, County Tract 4554, of several hundred new homes on urban sized lots. Additionally, the neighborhood includes a sizeable INS facility, and Rancho del Campo, and a small commercial center at Cameron's Corners, and the well known train museum and a truck museum. The area, during WW2 had ~~included~~ a prisoner of war camp housing Italians, and earlier, a US Army encampment.

9-12-06

[City of San Diego](#) [South Bay](#) [Imperial County](#)

Click on a parish or mission in your area for location, mass times and other information. C
[BACK] key on your browser to return to Parishes & Missions Search page.



Diocesan Schools

From: <http://www.diocese-sdiego.org/set.asp?link=directory.htm&in=Institutions>

School: **Holy Trinity School**
Principal: **Ms. Barbara Picco**
Grades: **(Pre-8)**
Address: **509 Ballard Street, El Cajon 92019**
Phone: **(619) 444-7529**
Fax: **(619) 444-3721**

*Estimated Travel Distance
and Time from Jamul, CA.*

12 miles / 27 minutes

School: **Santa Sophia Academy**
Principal: **Mrs. Karen Laaperi**
Grades: **(Pre-8)**
Address: **9806 San Juan Street, Spring Valley 91977**
Phone: **(619) 463-0488**
Fax: **(619) 668-5469**

12 miles / 27 minutes

School: **St. Kieran School**
Principal: **Mr. Peter Harris**
Grades: **(Pre-8)**
Address: **1347 Camillo Way, El Cajon 92021**
Phone: **(619) 588-6398**
Fax: **(619) 588-6382**

13 miles / 31 minutes

School: **St. Martin of Tours Academy**
Principal: **Ms. Nancy Ryan**
Grades: **(Pre-8)**
Address: **7708 El Cajon Blvd., La Mesa 91941**
Phone: **(619) 466-3241**
Fax: **(619) 466-0285**

15 miles / 31 minutes

School: **Our Lady of Grace School**
Principal: **Mrs. Susan Hause**
Grades: **(K-8)**
Address: **2766 Navajo Road, El Cajon 92020**
Phone: **(619) 466-0055**
Fax: **(619) 466-8994**

17 miles / 30 minutes

Estimated travel and distance from on-line tools

APPENDIX G

ITE 7TH EDITION TRIP GENERATION CALCUALTIONS

Summary of Trip Generation Calculation
 For 270 Students of Private School (K-12)
 March 28, 2005

















	Average Rate	Standard Deviation	Adjustment Factor	Driveway Volume
Avg. Weekday 2-Way Volume	2.48	0.00	1.00	670
7-9 AM Peak Hour Enter	0.48	0.00	1.00	130
7-9 AM Peak Hour Exit	0.31	0.00	1.00	84
7-9 AM Peak Hour Total	0.79	0.90	1.00	213
4-6 PM Peak Hour Enter	0.07	0.00	1.00	19
4-6 PM Peak Hour Exit	0.10	0.00	1.00	27
4-6 PM Peak Hour Total	0.17	0.41	1.00	46
AM Pk Hr, Generator, Enter	0.48	0.00	1.00	130
AM Pk Hr, Generator, Exit	0.31	0.00	1.00	84
AM Pk Hr, Generator, Total	0.79	0.90	1.00	213
PM Pk Hr, Generator, Enter	0.23	0.00	1.00	62
PM Pk Hr, Generator, Exit	0.32	0.00	1.00	86
PM Pk Hr, Generator, Total	0.55	0.74	1.00	149
Saturday 2-Way Volume	0.00	0.00	1.00	0
Saturday Peak Hour Enter	0.00	0.00	1.00	0
Saturday Peak Hour Exit	0.00	0.00	1.00	0
Saturday Peak Hour Total	0.00	0.00	1.00	0
Sunday 2-Way Volume	0.00	0.00	1.00	0
Sunday Peak Hour Enter	0.00	0.00	1.00	0
Sunday Peak Hour Exit	0.00	0.00	1.00	0
Sunday Peak Hour Total	0.00	0.00	1.00	0











Note: A zero indicates no data available.
 Source: Institute of Transportation Engineers
 Trip Generation, 7th Edition, 2003.










TRIP GENERATION BY MICROTRANS

















APPENDIX H

EXISTING + PROJECT INTERSECTION LEVEL OF SERVICE CALCUALTIONS










												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	2	0	1	21	1	32	2	41	41	47	33	2
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	2	0	1	22	1	34	3	43	43	49	35	2
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	240	227	36	207	207	65	37			86		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	240	227	36	207	207	65	37			86		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	97	100	97	100			97		
cM capacity (veh/h)	671	649	1037	730	666	999	1574			1510		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	3	57	89	86								
Volume Left	2	22	3	49								
Volume Right	1	34	43	2								
cSH	760	867	1574	1510								
Volume to Capacity	0.00	0.07	0.00	0.03								
Queue Length 95th (ft)	0	5	0	3								
Control Delay (s)	9.8	9.4	0.3	4.4								
Lane LOS	A	A	A	A								
Approach Delay (s)	9.8	9.4	0.3	4.4								
Approach LOS	A	A										
Intersection Summary												
Average Delay			4.1									
Intersection Capacity Utilization			21.1%		ICU Level of Service					A		
Analysis Period (min)			15									

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Volume (veh/h)	50	49	29	42	27	15
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	53	52	31	44	28	16
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type					None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			104		184	78
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			104		184	78
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			98		96	98
cM capacity (veh/h)			1487		789	982
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	104	75	44			
Volume Left	0	31	28			
Volume Right	52	0	16			
cSH	1700	1487	849			
Volume to Capacity	0.06	0.02	0.05			
Queue Length 95th (ft)	0	2	4			
Control Delay (s)	0.0	3.1	9.5			
Lane LOS		A	A			
Approach Delay (s)	0.0	3.1	9.5			
Approach LOS			A			
Intersection Summary						
Average Delay			2.9			
Intersection Capacity Utilization		20.5%		ICU Level of Service		A
Analysis Period (min)		15				

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Volume (veh/h)	39	47	16	60	73	20
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	41	49	17	63	77	21
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	223	48			80	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	223	48			80	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	94	95			95	
cM capacity (veh/h)	726	1020			1518	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	91	80	98			
Volume Left	41	0	77			
Volume Right	49	63	0			
cSH	862	1700	1518			
Volume to Capacity	0.11	0.05	0.05			
Queue Length 95th (ft)	9	0	4			
Control Delay (s)	9.7	0.0	6.0			
Lane LOS	A		A			
Approach Delay (s)	9.7	0.0	6.0			
Approach LOS	A					
Intersection Summary						
Average Delay		5.4				
Intersection Capacity Utilization		24.1%		ICU Level of Service		A
Analysis Period (min)		15				

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	11	8	1	19	11	43	3	29	6	35	37	8
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	12	8	1	20	12	45	4	31	6	37	39	8
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	210	162	43	164	163	34	47			37		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	210	162	43	164	163	34	47			37		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	98	99	100	97	98	96	100			98		
cM capacity (veh/h)	692	711	1027	777	711	1040	1560			1574		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	21	77	41	84								
Volume Left	12	20	4	37								
Volume Right	1	45	6	8								
cSH	711	898	1560	1574								
Volume to Capacity	0.03	0.09	0.00	0.02								
Queue Length 95th (ft)	2	7	0	2								
Control Delay (s)	10.2	9.4	0.8	3.3								
Lane LOS	B	A	A	A								
Approach Delay (s)	10.2	9.4	0.8	3.3								
Approach LOS	B	A										
Intersection Summary												
Average Delay			5.6									
Intersection Capacity Utilization			22.2%		ICU Level of Service					A		
Analysis Period (min)			15									

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↶			↷	↶	↷
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Volume (veh/h)	63	13	11	44	27	15
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	66	14	12	46	28	16
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None		
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			80		143	73
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			80		143	73
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		97	98
cM capacity (veh/h)			1518		844	989
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	80	58	44			
Volume Left	0	12	28			
Volume Right	14	0	16			
cSH	1700	1518	890			
Volume to Capacity	0.05	0.01	0.05			
Queue Length 95th (ft)	0	1	4			
Control Delay (s)	0.0	1.5	9.3			
Lane LOS		A	A			
Approach Delay (s)	0.0	1.5	9.3			
Approach LOS			A			
Intersection Summary						
Average Delay		2.7				
Intersection Capacity Utilization		19.6%		ICU Level of Service		A
Analysis Period (min)		15				

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Volume (veh/h)	14	17	15	10	13	21
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	15	18	16	11	14	22
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	71	21			26	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	71	21			26	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	98			99	
cM capacity (veh/h)	926	1056			1588	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	33	26	36			
Volume Left	15	0	14			
Volume Right	18	11	0			
cSH	993	1700	1588			
Volume to Capacity	0.03	0.02	0.01			
Queue Length 95th (ft)	3	0	1			
Control Delay (s)	8.7	0.0	2.8			
Lane LOS	A		A			
Approach Delay (s)	8.7	0.0	2.8			
Approach LOS	A					
Intersection Summary						
Average Delay		4.1				
Intersection Capacity Utilization		18.5%		ICU Level of Service		A
Analysis Period (min)		15				

APPENDIX I

EXISTING + PROJECT TWO-LANE HIGHWAY (HCM) CALCUALTIONS

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period AM
Highway SR-94
From/To Tecate Rd to Forest Gate Rd
Jurisdiction Caltrans
Analysis Year Existing+Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.8	mi	% Recreational vehicles	4	%
Terrain type	Rolling		% No-passing zones	100	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	187	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	0.71	
PCE for trucks, ET	2.5	
PCE for RVs, ER	1.1	
Heavy-vehicle adjustment factor,	0.824	
Two-way flow rate, (note-1) vp	363	pc/h
Highest directional split proportion (note-2)	243	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	4.3	mi/h
Average travel speed, ATS	47.7	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	0.77	
PCE for trucks, ET	1.8	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.899	
Two-way flow rate,(note-1) vp	307	pc/h
Highest directional split proportion (note-2)	206	
Base percent time-spent-following, BPTSF	23.7	%
Adj.for directional distribution and no-passing zones, fd/np	23.9	
Percent time-spent-following, PTSF	47.5	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.11	
Peak 15-min vehicle-miles of travel, VMT15	627	veh-mi
Peak-hour vehicle-miles of travel, VMT60	2207	veh-mi
Peak 15-min total travel time, TT15	13.2	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period PM
Highway SR-94
From/To Tecate Rd to Forest Gate Rd
Jurisdiction Caltrans
Analysis Year Existing+Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.8	mi	% Recreational vehicles	4	%
Terrain type	Rolling		% No-passing zones	100	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	182	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	0.71	
PCE for trucks, ET	2.5	
PCE for RVs, ER	1.1	
Heavy-vehicle adjustment factor,	0.824	
Two-way flow rate, (note-1) vp	354	pc/h
Highest directional split proportion (note-2)	237	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	4.3	mi/h
Average travel speed, ATS	47.8	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	0.77	
PCE for trucks, ET	1.8	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.899	
Two-way flow rate,(note-1) vp	299	pc/h
Highest directional split proportion (note-2)	200	
Base percent time-spent-following, BPTSF	23.1	%
Adj.for directional distribution and no-passing zones, fd/np	23.9	
Percent time-spent-following, PTSF	47.1	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.11	
Peak 15-min vehicle-miles of travel, VMT15	610	veh-mi
Peak-hour vehicle-miles of travel, VMT60	2148	veh-mi
Peak 15-min total travel time, TT15	12.8	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period AM
Highway SR-94
From/To Forest Gate Rd to Buckman Spr
Jurisdiction Caltrans
Analysis Year Existing + Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	1.4	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	138	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	172	pc/h
Highest directional split proportion (note-2)	115	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	1.6	mi/h
Average travel speed, ATS	51.8	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate, (note-1) vp	159	pc/h
Highest directional split proportion (note-2)	107	
Base percent time-spent-following, BPTSF	13.0	%
Adj. for directional distribution and no-passing zones, fd/np	21.7	
Percent time-spent-following, PTSF	34.8	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.05	
Peak 15-min vehicle-miles of travel, VMT15	55	veh-mi
Peak-hour vehicle-miles of travel, VMT60	193	veh-mi
Peak 15-min total travel time, TT15	1.1	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period PM
Highway SR-94
From/To Forest Gate Rd to Buckman Spr
Jurisdiction Caltrans
Analysis Year Existing + Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	1.4	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	171	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	213	pc/h
Highest directional split proportion (note-2)	143	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	2.0	mi/h
Average travel speed, ATS	51.2	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	197	pc/h
Highest directional split proportion (note-2)	132	
Base percent time-spent-following, BPTSF	15.9	%
Adj.for directional distribution and no-passing zones, fd/np	21.4	
Percent time-spent-following, PTSF	37.3	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.07	
Peak 15-min vehicle-miles of travel, VMT15	68	veh-mi
Peak-hour vehicle-miles of travel, VMT60	239	veh-mi
Peak 15-min total travel time, TT15	1.3	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period AM
Highway SR-94
From/To Buckman Springs to Sheridan Rd
Jurisdiction Caltrans
Analysis Year Existing + Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	0.8	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	187	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	233	pc/h
Highest directional split proportion (note-2)	133	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	2.1	mi/h
Average travel speed, ATS	50.9	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	215	pc/h
Highest directional split proportion (note-2)	123	
Base percent time-spent-following, BPTSF	17.2	%
Adj.for directional distribution and no-passing zones, fd/np	19.5	
Percent time-spent-following, PTSF	36.7	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.07	
Peak 15-min vehicle-miles of travel, VMT15	43	veh-mi
Peak-hour vehicle-miles of travel, VMT60	150	veh-mi
Peak 15-min total travel time, TT15	0.8	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period PM
Highway SR-94
From/To Buckman Springs to Sheridan Rd
Jurisdiction Caltrans
Analysis Year Existing+Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	0.8	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	144	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	180	pc/h
Highest directional split proportion (note-2)	103	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	1.7	mi/h
Average travel speed, ATS	51.7	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	166	pc/h
Highest directional split proportion (note-2)	95	
Base percent time-spent-following, BPTSF	13.6	%
Adj.for directional distribution and no-passing zones, fd/np	19.6	
Percent time-spent-following, PTSF	33.1	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.06	
Peak 15-min vehicle-miles of travel, VMT15	33	veh-mi
Peak-hour vehicle-miles of travel, VMT60	115	veh-mi
Peak 15-min total travel time, TT15	0.6	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period AM
Highway SR-94
From/To Sheridan Rd to White Star
Jurisdiction Caltrans
Analysis Year Existing + Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.3	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	165	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	206	pc/h
Highest directional split proportion (note-2)	117	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	1.9	mi/h
Average travel speed, ATS	51.3	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate, (note-1) vp	190	pc/h
Highest directional split proportion (note-2)	108	
Base percent time-spent-following, BPTSF	15.4	%
Adj. for directional distribution and no-passing zones, fd/np	19.5	
Percent time-spent-following, PTSF	34.9	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.06	
Peak 15-min vehicle-miles of travel, VMT15	530	veh-mi
Peak-hour vehicle-miles of travel, VMT60	1865	veh-mi
Peak 15-min total travel time, TT15	10.3	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period PM
Highway SR-94
From/To Sheridan Rd to White Star
Jurisdiction Caltrans
Analysis Year Existing + Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.3	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	138	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	172	pc/h
Highest directional split proportion (note-2)	98	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	1.6	mi/h
Average travel speed, ATS	51.8	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate, (note-1) vp	159	pc/h
Highest directional split proportion (note-2)	91	
Base percent time-spent-following, BPTSF	13.0	%
Adj. for directional distribution and no-passing zones, fd/np	19.6	
Percent time-spent-following, PTSF	32.6	%

Level of Service and Other Performance Measures

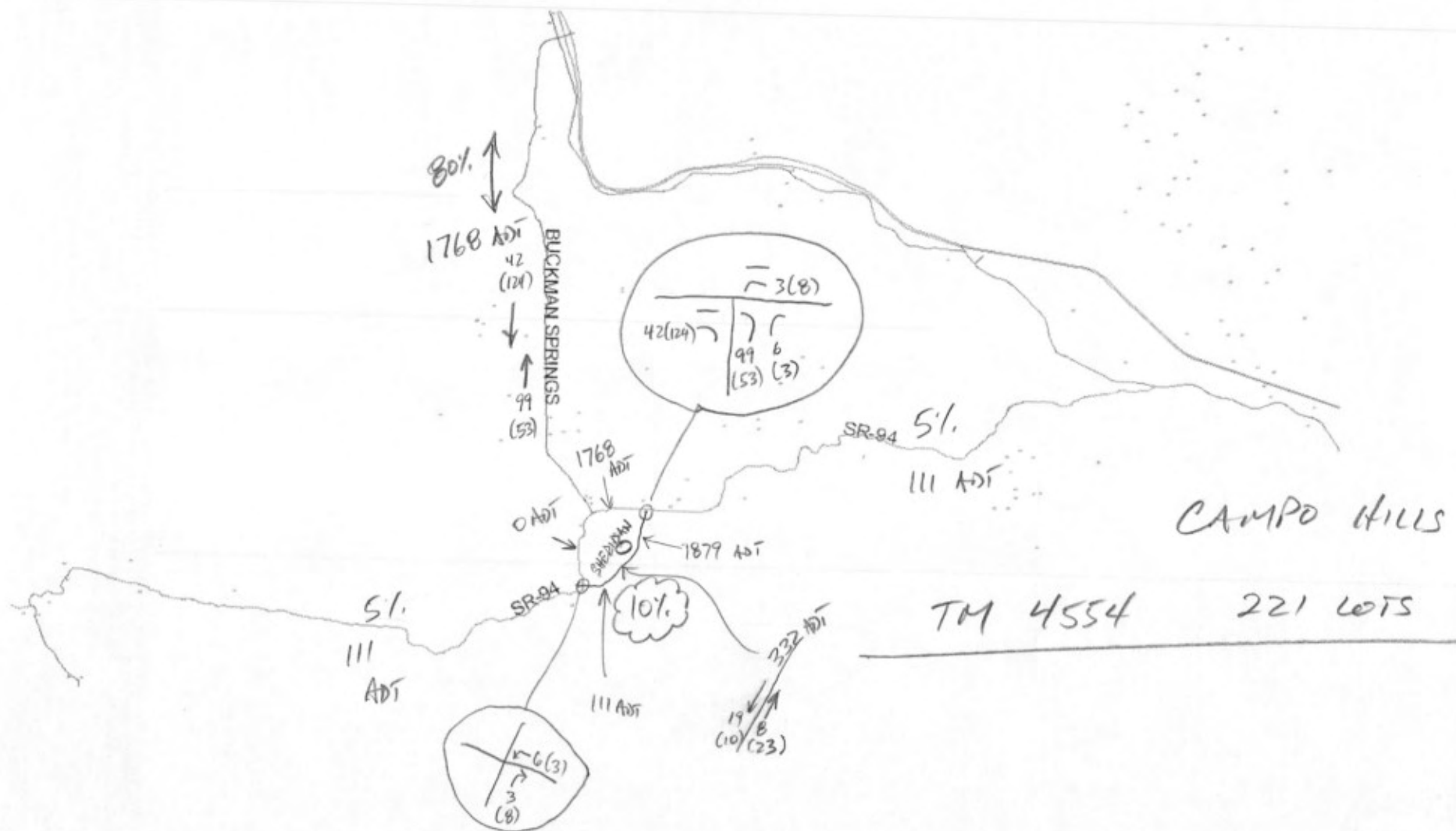
Level of service, LOS	B	
Volume to capacity ratio, v/c	0.05	
Peak 15-min vehicle-miles of travel, VMT15	443	veh-mi
Peak-hour vehicle-miles of travel, VMT60	1559	veh-mi
Peak 15-min total travel time, TT15	8.5	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

APPENDIX J

CUMULATIVE PROJECT INFORMATION



Proposed Land Use	Rate	Size & Units	ADT	%	Split	AM		%	Split	PM	
						IN	OUT			IN	OUT
Residential	10 /DU	221 DU	2,210	8%	0.3 0.7	53	124	10%	0.7 0.3	155	66

Source: SANDAG Brief Guide of Vehicular Traffic Generation Rates for the San Diego Region, April 2002.

CUMULATIVE TRAFFIC DATA SHEET

The following table has been compiled by County DPLU staff to assist in the preparation of cumulative traffic analysis for discretionary projects within the County of San Diego. The following information is requested by the County of San Diego for an interim use by traffic consultants while a public-interface of County of San Diego permit tracking and GIS applications is being programmed.

Please fill out all of the information below based on the case file pulled for cumulative analysis preparation. Once the form has been completed, County staff will provide you a photocopy of the form for your records and place the original in the "Cumulative Traffic Data Reference Binder" for inclusion into a binder for future reference.

Date Prepared: <u>2/8/05</u>	Prepared by: <u>Sami Raya</u>	858-694-3733
Today's date	Print your name (and firm name, if applicable)	Phone #
Project Name or Address: <u>Vaughan TM; 30069 Canvasback Drive</u>		
Community Planning Area: <u>Mountain Empire</u>		
Thomas Brother's page and location: <u>1297-G5; Easternmost terminal end Canvasback Drive</u>		
Case Numbers (e.g., TM, TPM, MUP, STP, ER, etc.): <u>TM 5417/ ER04-21-007</u>		
Assessor Parcel Number(s): <u>606-131-15; 606-141-23</u>		
Residential Lots: <u>13</u> Lots	Commercial Square Footage: <u> </u> s.f	Other?(explain below)
<p>Project Description - Please summarize type of project – residential, commercial, school, church, industrial, etc. (If <i>residential</i>, number of lots and type of residential – apartment, condo, townhome, single-family; if <i>commercial</i>, number of square feet of building space and on-site parking; If <i>school, church or other non-traditional</i> traffic generator, please describe the project's enrollment/capacity/attendance or any other characteristic in which traffic generation can be derived)</p> <p>The project proposes a major subdivision of 81.24 acres (gross) into 13 lots that will measure between 5 and 8.98 acres (net). The lots will be developed with single-family residences.</p>		
<p>Public Roadway(s) Providing direct access to the project site (This includes <i>all</i> roadways that the future occupants would use to directly enter the site.) From Buckman Springs Road, to Oak Drive, to White Goose Road, to Canvasback Drive.</p>		
Project Traffic Consultant: <u>N/A</u>		Final Traffic Report Avail. (if yes, please check) <input type="checkbox"/>

As noted above, this information, as well as that information received from other consultants completing cumulative traffic analysis, will be available for future reference at the Project Processing Counter in the *Cumulative Traffic Data Reference Binder*.

CUMULATIVE TRAFFIC DATA SHEET

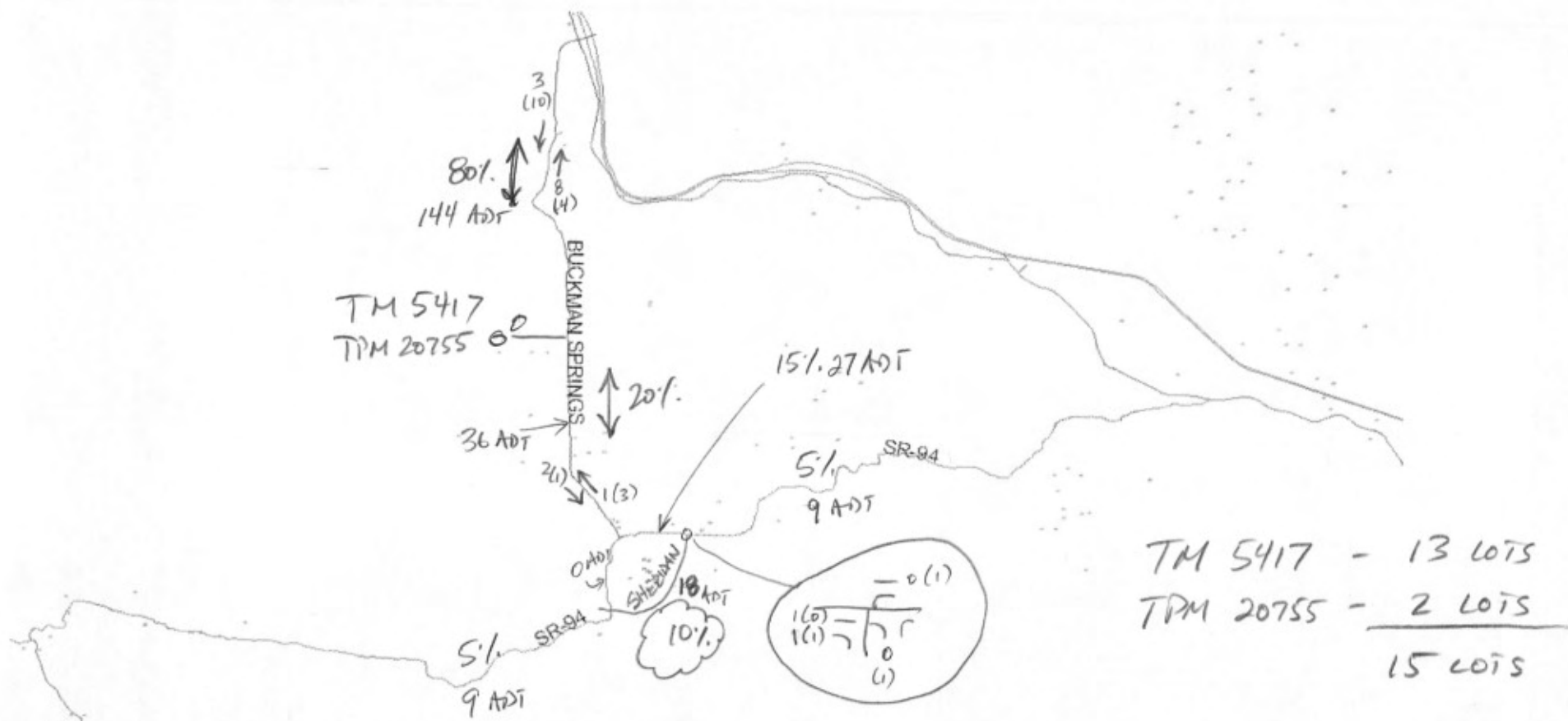
The following table has been compiled by County DPLU staff to assist in the preparation of cumulative traffic analysis for discretionary projects within the County of San Diego. The following information is requested by the County of San Diego for an interim use by traffic consultants while a public-interface of County of San Diego permit tracking and GIS applications is being programmed.

Please fill out all of the information below based on the case file pulled for cumulative analysis preparation. Once the form has been completed, County staff will provide you a photocopy of the form for your records and place the original in the "Cumulative Traffic Data Reference Binder" for inclusion into a binder for future reference.

Date Prepared: <u>11/10/04</u>		Prepared by: <u>Jarrett Ramaiya/DPLU</u>		(858) 694-3015
Today's date		Print your name (and firm name, if applicable)		Phone #
Project Name or Address: <u>McClintock TPM/Oak Drive at Basso Road</u>				
Community Planning Area: <u>Mountain Empire CPG</u>				
Thomas Brother's page and location: <u>1297 5/F</u>				
Case Numbers (e.g., TM, TPM, MUP, STP, ER, etc.): <u>TPM 20755</u>				
Assessor Parcel Number(s): <u>606-140-29</u>				
Residential Lots: 2 (proposed) Lots		Commercial Square Footage: _____ s.f		Other?(explain below)
Project Description - Please summarize type of project - residential, commercial, school, church, industrial, etc. (If residential, number of lots and type of residential - apartment, condo, townhome, single-family; if commercial, number of square feet of building space and on-site parking; If school, church or other non-traditional traffic generator, please describe the project's enrollment/capacity/attendance or any other characteristic in which traffic generation can be derived) The project proposes a minor subdivision of 10.0 gross acres into 2 residential parcels of 4.15 ac and 4.56 acres net. The subdivision is located on Basso Road in the Campo/Lake Morena Community. Regional Category: (RDA) Rural Development Area, Land Use Designator: (18) Multiple Rural Use, Planning Area: Mountain Empire, Planning Group: Campo/Lake Morena Sponsor Group, Use Regulation: S92(4 acre minimum lot size).				
Public Roadway(s) Providing direct access to the project site (This includes all roadways that the future occupants would use to directly enter the site.) <u>Oak Drive & Basso Road</u>				
Project Traffic Consultant: <u>Not yet assigned</u>			Final Traffic Report Avail. (if yes, please check) <input type="checkbox"/>	

As noted above, this information, as well as that information received from other consultants completing cumulative traffic analysis, will be available for future reference at the Project Processing Counter in the *Cumulative Traffic Data Reference Binder*.

Please return the master to Project Processing Staff for insertion into the Cumulative Traffic
 St. Adelaide Catholic Church Traffic Study Appendix



Proposed Land Use	Rate	Size & Units	ADT	%	Split	AM		%	Split	PM	
						IN	OUT			IN	OUT
Residential	12 /DU	15 DU	180	8%	0.3 0.7	4	10	10%	0.7 0.3	13	5

Source: SANDAG Brief Guide of Vehicular Traffic Generation Rates for the San Diego Region, April 2002.

CUMULATIVE TRAFFIC DATA SHEET

The following table has been compiled by County DPLU staff to assist in the preparation of cumulative traffic analysis for discretionary projects within the County of San Diego. The following information is requested by the County of San Diego for an interim use by traffic consultants while a public-interface of County of San Diego permit tracking and GIS applications is being programmed.

Please fill out all of the information below based on the case file pulled for cumulative analysis preparation. Once the form has been completed, County staff will provide you a photocopy of the form for your records and place the original in the "Cumulative Traffic Data Reference Binder" for inclusion into a binder for future reference.

Date Prepared:	November 16, 2004	Prepared by:	Daniella Rosenberg	858 694-3016
	Today's date		Print your name (and firm name, if applicable)	Phone #
Project Name or Address: Legacy Ranch Estates				
Community Planning Area: Campo				
Thomas Brother's page and location: Page 1298 F5				
Case Numbers (e.g., TM, TPM, MUP, STP, ER, etc.): TM 53071, ER 04-21-001				
Assessor Parcel Number(s): 606-180-04				
Residential Lots: 9 Lots		Commercial Square Footage: s.f		Other?
Project Description - Please summarize type of project - residential, commercial, school, church, industrial, etc. The proposed project consists of an nine-lot subdivision on approximately 79.23 acres and a remainder parcel of 80.77 acres. The entire ownership is 160.00 acres.				
Public Roadway(s) Providing direct access to the project site (This includes all roadways that the future occupants would use to directly enter the site.) La Posta Road				
Project Traffic Consultant: N/A			Final Traffic Report Avail. <input type="checkbox"/>	

As noted above, this information, as well as that information received from other consultants completing cumulative traffic analysis, will be available for future reference at the Project Processing Counter in the *Cumulative Traffic Data Reference Binder*.

Please return the master to Project Processing Staff for insertion into the Cumulative Traffic Data Reference Binder.



Proposed Land Use	Rate	Size & Units	ADT	%	Split	AM		%	Split	PM	
						IN	OUT			IN	OUT
Residential	12 /DU	9 DU	108	8%	0.3 0.7	3	6	10%	0.7 0.3	8	3

Source: SANDAG Brief Guide of Vehicular Traffic Generation Rates for the San Diego Region, April 2002.

CUMULATIVE TRAFFIC DATA SHEET

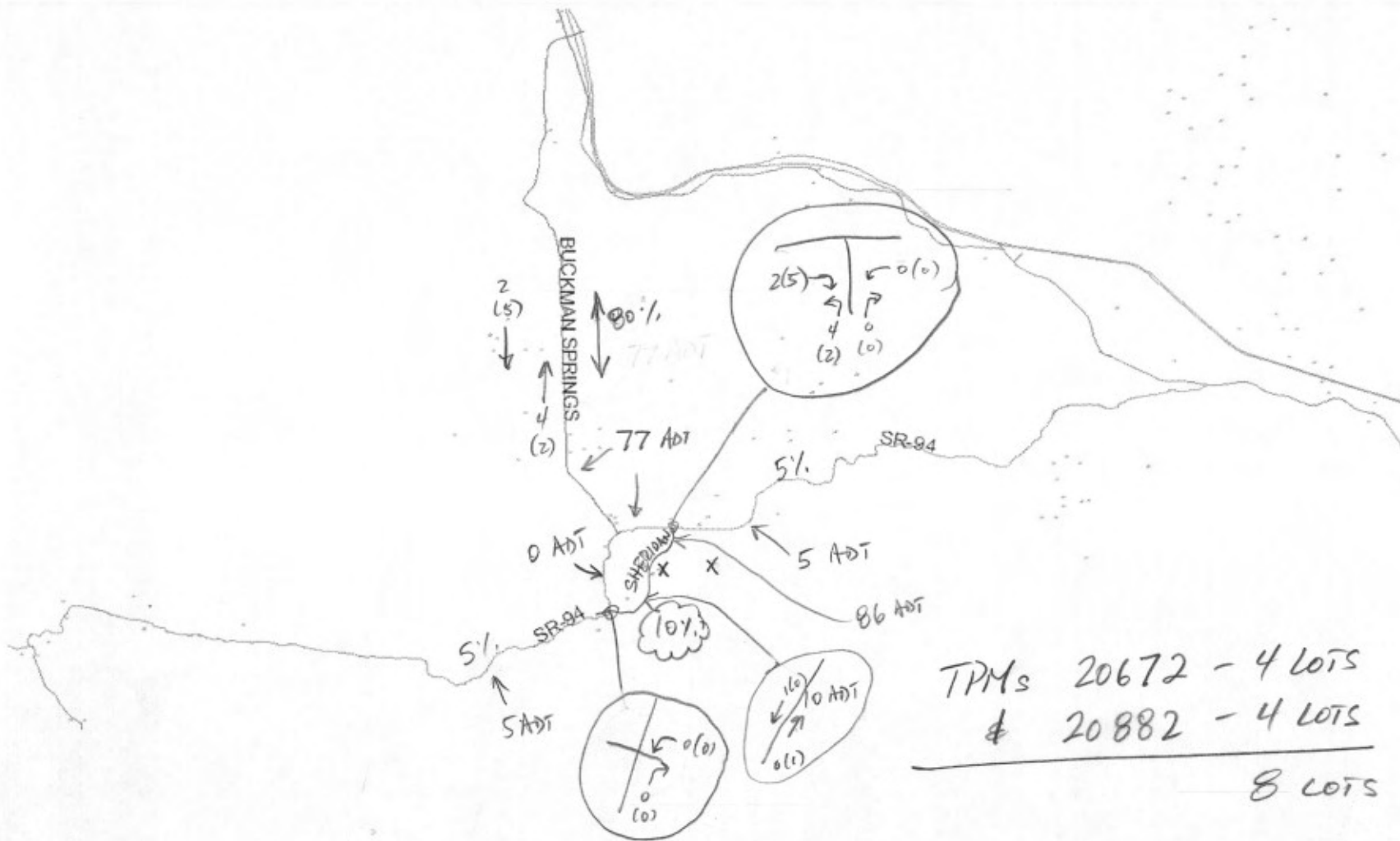
The following table has been compiled by County DPLU staff to assist in the preparation of cumulative traffic analysis for discretionary projects within the County of San Diego. The following information is requested by the County of San Diego for an interim use by traffic consultants while a public-interface of County of San Diego permit tracking and GIS applications is being programmed.

Please fill out all of the information below based on the case file pulled for cumulative analysis preparation. Once the form has been completed, County staff will provide you a photocopy of the form for your records and place the original in the "Cumulative Traffic Data Reference Binder" for inclusion into a binder for future reference.

Date Prepared: <u>11/2/04</u>	Prepared by: <u>Alyssa Maxson, DPLU</u>	(858)694-3737
Today's date	Print your name (and firm name, if applicable)	Phone #
Project Name or Address: <u>Tortora TPM</u>		
Community Planning Area: <u>Mountain Empire Community Planning Area</u>		
Thomas Brother's page and location: <u>1318 D/6</u>		
Case Numbers (e.g., TM, TPM, MUP, STP, ER, etc.): <u>TPM 20882; ER 04-21-002</u>		
Assessor Parcel Number(s): <u>655-130-22-00</u>		
Residential Lots: <u>4</u> Lots	Commercial Square Footage: <u> </u> s.f	Other?(explain below)
<p>Project Description - Please summarize type of project - residential, commercial, school, church, industrial, etc. (If residential, number of lots and type of residential - apartment, condo, townhome, single-family; if commercial, number of square feet of building space and on-site parking; If school, church or other non-traditional traffic generator, please describe the project's enrollment/capacity/attendance or any other characteristic in which traffic generation can be derived)</p> <p>The proposed project is a subdivision of a parcel into four residential lots. A single-family residence currently exists on the project site and would remain with this project. Three additional single-family residential lots would be created.</p>		
<p>Public Roadway(s) Providing direct access to the project site (This includes all roadways that the future occupants would use to directly enter the site.)</p> <p>Highway 94 to Meanwhile Ranch Road</p>		
Project Traffic Consultant:		Final Traffic Report Avail. (if yes, please check) <input type="checkbox"/>

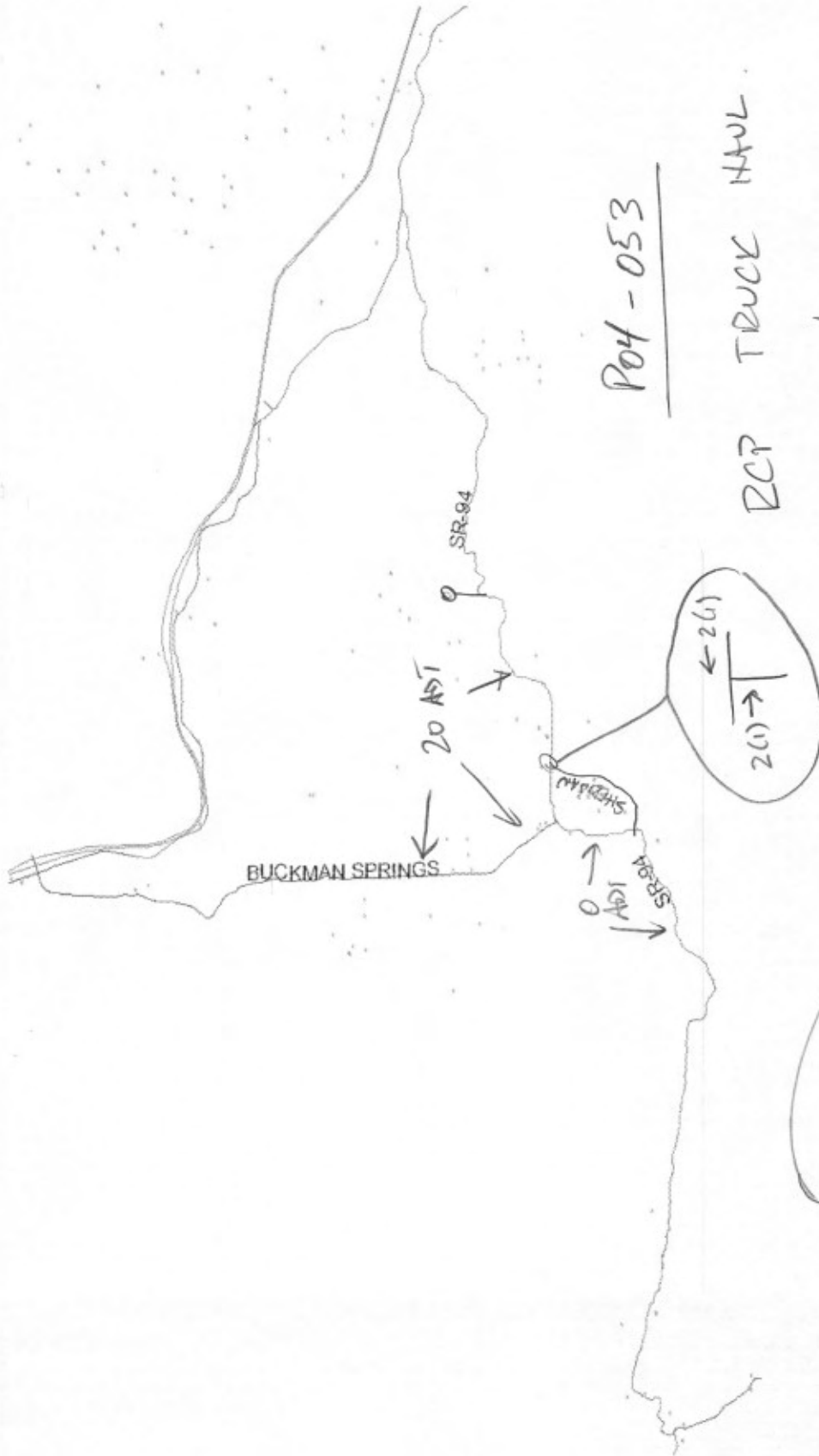
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Please return the master to Project Processing Staff for insertion into the Cumulative Traffic Data Reference Binder.



Proposed Land Use	Rate	Size & Units	ADT	%	Split	AM		%	Split	PM	
						IN	OUT			IN	OUT
Residential	12 /DU	8 DU	96	8%	0.3 0.7	2	5	10%	0.7 0.3	7	3

Source: SANDAG Brief Guide of Vehicular Traffic Generation Rates for the San Diego Region, April 2002.



20 ADT / DAY

ASSUME HOURS OF CREATION 8-5.

20 ADT / 9 HRS = 2.2 Vehicles / hr.

AM		PM	
IN	OUT	IN	OUT
2	1	1	2

Planning Area:

Mountain Empire
LAKE MORENA/CAMPO

Case Number:

P04-053
RP04-004

CUMULATIVE TRAFFIC DATA SHEET

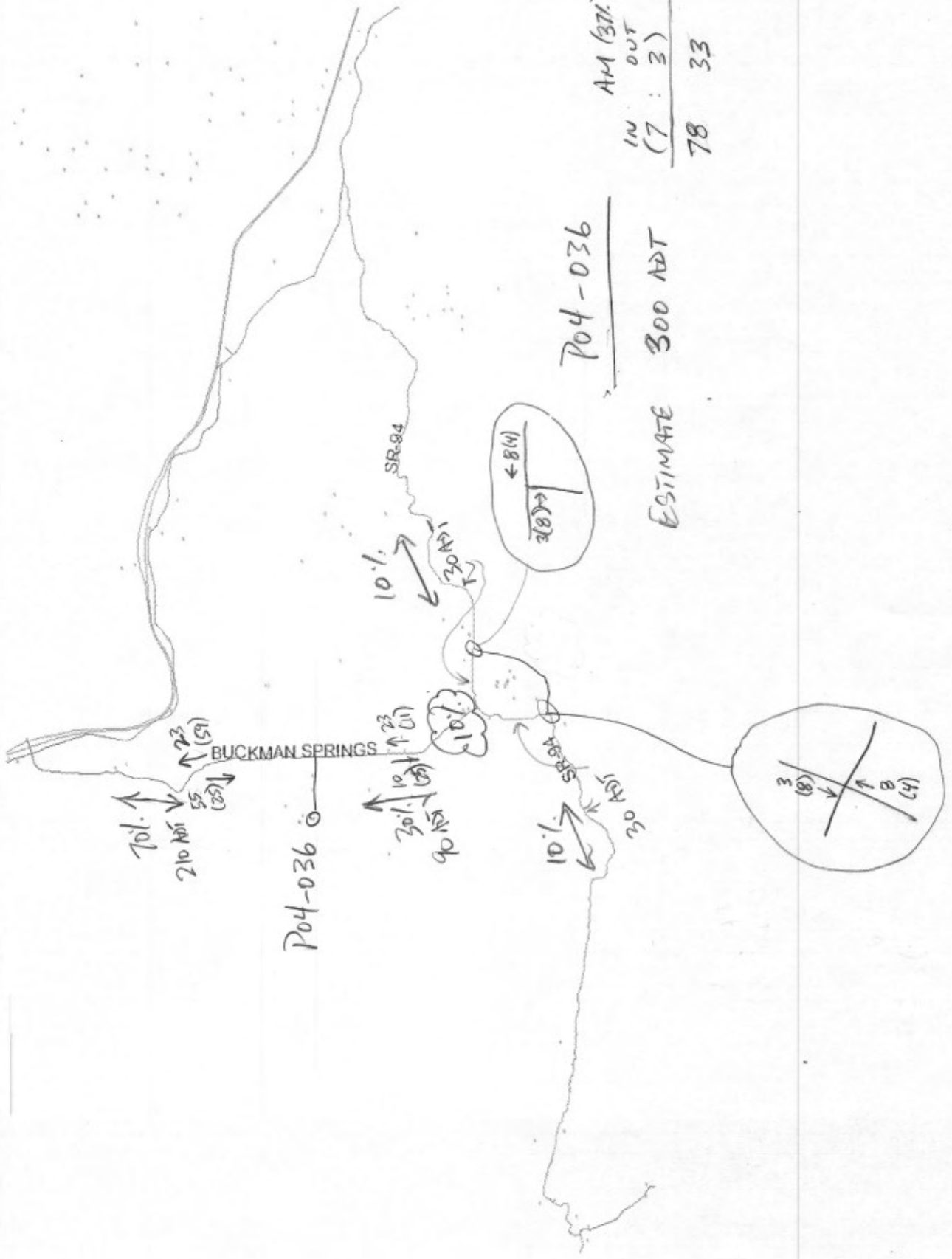
The following table has been compiled by County DPLU staff to assist in the preparation of cumulative traffic analysis for discretionary projects within the County of San Diego. The following information is requested by the County of San Diego for an interim use by traffic consultants while a public-interface of County of San Diego permit tracking and GIS applications is being programmed.

Please fill out all of the information below based on the case file pulled for cumulative analysis preparation. Once the form has been completed, County staff will provide you a photocopy of the form for your records and place the original in the "Cumulative Traffic Data Reference Binder" for inclusion into a binder for future reference.

Date Prepared: <u>1/5/05</u>		Prepared by: <u>A. Maxson</u>	<u>x3737</u>
Today's date		Print your name (and firm name, if applicable)	Phone #
Project Name or Address: <u>RCP - Circle F Ranch</u>			
Community Planning Area: <u>Mountain Empire</u>			
Thomas Brother's page and location: <u>1298 H/6, J/6, G/7, H/7, J/7; 1318 H/1, J/1, H/2, J/2</u>			
Case Numbers (e.g., TM, TPM, MUP, STP, ER, etc.): <u>P04-053, RP04-004; ER 04-21-003</u>			
Assessor Parcel Number(s):			
Residential Lots: <u> </u> Lots	Commercial Square Footage: <u> </u> s.f.	Other?(explain below)	
Project Description - Please summarize type of project - residential, commercial, school, church, industrial, etc. (If residential, number of lots and type of residential - apartment, condo, townhome, single-family; if commercial, number of square feet of building space and on-site parking; If school, church or other non-traditional traffic generator, please describe the project's enrollment/capacity/attendance or any other characteristic in which traffic generation can be derived) <u>Sand Extraction operation with approximately 20 haul trucks per day. (employee total not known at this time).</u>			
Public Roadway(s) Providing direct access to the project site (This includes all roadways that the future occupants would use to directly enter the site.) <u>SR 94 - Proposed Route is I-8 to Buckman Springs Rd to SR 94</u>			
Project Traffic Consultant:		Final Traffic Report Avail. (if yes, please check) <input type="checkbox"/>	

As noted above, this information, as well as that information received from other consultants completing cumulative traffic analysis, will be available for future reference at the Project Processing Counter in the *Cumulative Traffic Data Reference Binder*.

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P04-036

ESTIMATE 300 ADT

AM (37%)		PM (40%)	
IN	OUT	IN	OUT
(7 : 3)		(3 : 7)	
78	33	36	84

CUMULATIVE TRAFFIC DATA SHEET

The following table has been compiled by County DPLU staff to assist in the preparation of cumulative traffic analysis for discretionary projects within the County of San Diego. The following information is requested by the County of San Diego for an interim use by traffic consultants while a public-interface of County of San Diego permit tracking and GIS applications is being programmed.

Please fill out all of the information below based on the case file pulled for cumulative analysis preparation. Once the form has been completed, County staff will provide you a photocopy of the form for your records and place the original in the "Cumulative Traffic Data Reference Binder" for inclusion into a binder for future reference.

Date Prepared: <u>11/8/2004</u>	Prepared by: <u>Bob Forsythe</u> <u>David Sibbet</u>	(858) 694-3680 3856
Today's date	Print your name (and firm name, if applicable)	Phone #
Project Name or Address: <u>A Children's Village, 1880 Lake Morena Drive</u>		
Community Planning Area: <u>Mountain Empire Subregion</u>		
Thomas Brother's page and location: <u>1317 G2, in between Buckman Springs and Oak Drive.</u>		
Case Numbers (e.g., TM, TPM, MUP, STP, ER, etc.): <u>P04-036</u>		
Assessor Parcel Numbers: <u>607-10028</u>		
Residential Lots: 0 Lots	Commercial Square Footage: 0 s.f.	Other? Civic Use
<p>Project Description – The Major Use Permit is for three uses on the site: a group care facility for 200 foster care children, an on-site school (K through 12), and a interdenominational church. There will also be several accessory uses on the property, including: a water reclamation facility, a reservoir, a water tank, parking lots, pasture land, stables, barns, corrals, the bunk house, a caretakers house, a guest house, a basketball court, a tennis court, a / soccer field, and a baseball field.</p> <p>The housing units will be located in five groups around the perimeter of the northern side of the property. Each group of units will include 5 separate dwelling units each. Each unit will house up to 8 foster children and a live-in married couple. A 10-space parking lot will be constructed near every group of housing units. The live-in couple's biological children may also stay at the facility. The director and his family along with various guests will stay on the property at any given time. A total occupancy of 270 people is anticipated.</p> <p>The school will cover approximately the central 7 acres of the property. The school will accommodate the entire children population. 15 teachers and 5 support staff will work at the school. Recreational areas, courts and fields will be located south of the school. All the proposed buildings will use a rural ranch-like architectural design to maintain the rural character of the area. The interdenominational chapel near the main entrance of the property will serve on-site and the greater Moreno community. The seating capacity of the chapel will be 424 persons. The chapel will be used for weekly religious services and occasional weddings.</p> <p>Three gated entrances off Lake Moreno Drive will provide adequate everyday and emergency access. A total of 122 parking spaces will be available on the property and provide adequate parking for all the uses.</p>		

Public Roadway(s) Providing direct access to the project site
Access will be provided via Lake Morena Drive

Project Traffic Consultant: Darnell and Assocs.

















Final Traffic Report Avail. (if yes, please check) ☐











As noted above, this information, as well as that information received from other consultants completing cumulative traffic analysis, will be available for future reference at the Project Processing Counter in the *Cumulative Traffic Data Reference Binder*.










Please return the master to Project Processing Staff for insertion into the Cumulative Traffic Data Reference Binder.

















APPENDIX K











EXISTING + PROJECT + CUMULATIVE INTERSECTION LEVEL OF SERVICE CALCUALTIONS










												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	2	0	1	27	1	32	2	49	44	47	36	2
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	2	0	1	28	1	34	4	52	46	74	38	2
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	305	294	39	272	272	75	40			98		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	305	294	39	272	272	75	40			98		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	96	100	97	100			95		
cM capacity (veh/h)	600	585	1033	653	602	987	1570			1495		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	3	63	102	114								
Volume Left	2	28	4	74								
Volume Right	1	34	46	2								
cSH	697	795	1570	1495								
Volume to Capacity	0.00	0.08	0.00	0.05								
Queue Length 95th (ft)	0	6	0	4								
Control Delay (s)	10.2	9.9	0.3	5.0								
Lane LOS	B	A	A	A								
Approach Delay (s)	10.2	9.9	0.3	5.0								
Approach LOS	B	A										
Intersection Summary												
Average Delay			4.5									
Intersection Capacity Utilization			22.9%		ICU Level of Service					A		
Analysis Period (min)			15									

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Volume (veh/h)	58	94	29	57	130	22
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	61	99	46	60	137	23
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			160		262	111
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			160		262	111
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			97		81	98
cM capacity (veh/h)			1419		703	943
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	160	106	160			
Volume Left	0	46	137			
Volume Right	99	0	23			
cSH	1700	1419	730			
Volume to Capacity	0.09	0.03	0.22			
Queue Length 95th (ft)	0	2	21			
Control Delay (s)	0.0	3.4	11.3			
Lane LOS		A	B			
Approach Delay (s)	0.0	3.4	11.3			
Approach LOS			B			
Intersection Summary						
Average Delay		5.1				
Intersection Capacity Utilization		33.8%		ICU Level of Service		A
Analysis Period (min)		15				

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Volume (veh/h)	39	47	25	60	73	41
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	41	49	26	63	154	43
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	408	58			89	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	408	58			89	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	92	95			90	
cM capacity (veh/h)	538	1008			1506	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	91	89	197			
Volume Left	41	0	154			
Volume Right	49	63	0			
cSH	722	1700	1506			
Volume to Capacity	0.13	0.05	0.10			
Queue Length 95th (ft)	11	0	9			
Control Delay (s)	10.7	0.0	6.2			
Lane LOS	B		A			
Approach Delay (s)	10.7	0.0	6.2			
Approach LOS	B					
Intersection Summary						
Average Delay		5.8				
Intersection Capacity Utilization		29.2%		ICU Level of Service		A
Analysis Period (min)		15				

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	11	8	1	22	11	43	3	33	14	35	45	8
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	12	8	1	23	12	45	6	35	22	55	47	8
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	272	232	52	226	225	46	56			57		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	272	232	52	226	225	46	56			57		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	98	99	100	97	98	96	100			96		
cM capacity (veh/h)	623	642	1016	700	648	1024	1549			1548		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	21	80	63	111								
Volume Left	12	23	6	55								
Volume Right	1	45	22	8								
cSH	643	840	1549	1548								
Volume to Capacity	0.03	0.10	0.00	0.04								
Queue Length 95th (ft)	3	8	0	3								
Control Delay (s)	10.8	9.7	0.8	3.8								
Lane LOS	B	A	A	A								
Approach Delay (s)	10.8	9.7	0.8	3.8								
Approach LOS	B	A										
Intersection Summary												
Average Delay			5.4									
Intersection Capacity Utilization			23.8%			ICU Level of Service				A		
Analysis Period (min)			15									

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Volume (veh/h)	78	143	11	53	83	18
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	82	151	17	56	87	19
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type					None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			233		248	157
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			233		248	157
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		88	98
cM capacity (veh/h)			1335		731	888
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	233	73	106			
Volume Left	0	17	87			
Volume Right	151	0	19			
cSH	1700	1335	755			
Volume to Capacity	0.14	0.01	0.14			
Queue Length 95th (ft)	0	1	12			
Control Delay (s)	0.0	1.9	10.6			
Lane LOS		A	B			
Approach Delay (s)	0.0	1.9	10.6			
Approach LOS			B			
Intersection Summary						
Average Delay		3.1				
Intersection Capacity Utilization		30.2%		ICU Level of Service		A
Analysis Period (min)		15				

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Volume (veh/h)	14	17	40	10	13	32
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	15	18	42	11	21	34
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	122	47			53	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	122	47			53	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	98			99	
cM capacity (veh/h)	862	1022			1553	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	33	53	54			
Volume Left	15	0	21			
Volume Right	18	11	0			
cSH	943	1700	1553			
Volume to Capacity	0.03	0.03	0.01			
Queue Length 95th (ft)	3	0	1			
Control Delay (s)	9.0	0.0	2.8			
Lane LOS	A		A			
Approach Delay (s)	9.0	0.0	2.8			
Approach LOS	A					
Intersection Summary						
Average Delay		3.2				
Intersection Capacity Utilization		19.4%		ICU Level of Service		A
Analysis Period (min)		15				

APPENDIX L

EXISTING + PROJECT + CUMULATIVE TWO-LANE HIGHWAY (HCM) CALCUALTIONS

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period AM
Highway SR-94
From/To Tecate Rd to Forest Gate Rd
Jurisdiction Caltrans
Analysis Year Existing+Project+Cumulative
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.8	mi	% Recreational vehicles	4	%
Terrain type	Rolling		% No-passing zones	100	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	196	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	0.71	
PCE for trucks, ET	2.5	
PCE for RVs, ER	1.1	
Heavy-vehicle adjustment factor,	0.824	
Two-way flow rate, (note-1) vp	381	pc/h
Highest directional split proportion (note-2)	255	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	4.4	mi/h
Average travel speed, ATS	47.4	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	0.77	
PCE for trucks, ET	1.8	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.899	
Two-way flow rate, (note-1) vp	322	pc/h
Highest directional split proportion (note-2)	216	
Base percent time-spent-following, BPTSF	24.7	%
Adj. for directional distribution and no-passing zones, fd/np	23.7	
Percent time-spent-following, PTSF	48.4	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.12	
Peak 15-min vehicle-miles of travel, VMT15	657	veh-mi
Peak-hour vehicle-miles of travel, VMT60	2313	veh-mi
Peak 15-min total travel time, TT15	13.8	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period PM
Highway SR-94
From/To Tecate Rd to Forest Gate Rd
Jurisdiction Caltrans
Analysis Year Existing+Project+Cumulative
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.8	mi	% Recreational vehicles	4	%
Terrain type	Rolling		% No-passing zones	100	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	202	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	0.71	
PCE for trucks, ET	2.5	
PCE for RVs, ER	1.1	
Heavy-vehicle adjustment factor,	0.824	
Two-way flow rate, (note-1) vp	392	pc/h
Highest directional split proportion (note-2)	263	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	4.5	mi/h
Average travel speed, ATS	47.3	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	0.77	
PCE for trucks, ET	1.8	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.899	
Two-way flow rate,(note-1) vp	331	pc/h
Highest directional split proportion (note-2)	222	
Base percent time-spent-following, BPTSF	25.2	%
Adj.for directional distribution and no-passing zones, fd/np	23.6	
Percent time-spent-following, PTSF	48.9	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.12	
Peak 15-min vehicle-miles of travel, VMT15	677	veh-mi
Peak-hour vehicle-miles of travel, VMT60	2384	veh-mi
Peak 15-min total travel time, TT15	14.3	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period AM
Highway SR-94
From/To Forest Gate Rd to Buckman Spr
Jurisdiction Caltrans
Analysis Year Existing + Project + Cumulative
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	1.4	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	149	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	186	pc/h
Highest directional split proportion (note-2)	125	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	1.8	mi/h
Average travel speed, ATS	51.6	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	172	pc/h
Highest directional split proportion (note-2)	115	
Base percent time-spent-following, BPTSF	14.0	%
Adj.for directional distribution and no-passing zones, fd/np	21.6	
Percent time-spent-following, PTSF	35.6	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.06	
Peak 15-min vehicle-miles of travel, VMT15	59	veh-mi
Peak-hour vehicle-miles of travel, VMT60	209	veh-mi
Peak 15-min total travel time, TT15	1.1	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period PM
Highway SR-94
From/To Forest Gate Rd to Buckman Spr
Jurisdiction Caltrans
Analysis Year Existing + Project + Cumulative
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	1.4	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	183	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	228	pc/h
Highest directional split proportion (note-2)	153	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	2.1	mi/h
Average travel speed, ATS	51.0	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	211	pc/h
Highest directional split proportion (note-2)	141	
Base percent time-spent-following, BPTSF	16.9	%
Adj.for directional distribution and no-passing zones, fd/np	21.2	
Percent time-spent-following, PTSF	38.1	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.07	
Peak 15-min vehicle-miles of travel, VMT15	73	veh-mi
Peak-hour vehicle-miles of travel, VMT60	256	veh-mi
Peak 15-min total travel time, TT15	1.4	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period AM
Highway SR-94
From/To Buckman Springs to Sheridan Rd
Jurisdiction Caltrans
Analysis Year Existing + Project + Cumulative
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	0.8	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	358	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	447	pc/h
Highest directional split proportion (note-2)	255	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	3.0	mi/h
Average travel speed, ATS	48.3	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate, (note-1) vp	413	pc/h
Highest directional split proportion (note-2)	235	
Base percent time-spent-following, BPTSF	30.4	%
Adj.for directional distribution and no-passing zones, fd/np	19.0	
Percent time-spent-following, PTSF	49.5	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.14	
Peak 15-min vehicle-miles of travel, VMT15	81	veh-mi
Peak-hour vehicle-miles of travel, VMT60	286	veh-mi
Peak 15-min total travel time, TT15	1.7	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period PM
Highway SR-94
From/To Buckman Springs to Sheridan Rd
Jurisdiction Caltrans
Analysis Year Existing + Project + Cumulative
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	0.8	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	354	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	442	pc/h
Highest directional split proportion (note-2)	252	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	3.0	mi/h
Average travel speed, ATS	48.4	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	408	pc/h
Highest directional split proportion (note-2)	233	
Base percent time-spent-following, BPTSF	30.1	%
Adj.for directional distribution and no-passing zones, fd/np	19.1	
Percent time-spent-following, PTSF	49.2	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.14	
Peak 15-min vehicle-miles of travel, VMT15	80	veh-mi
Peak-hour vehicle-miles of travel, VMT60	283	veh-mi
Peak 15-min total travel time, TT15	1.7	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period AM
Highway SR-94
From/To Sheridan Rd to White Star
Jurisdiction Caltrans
Analysis Year Existing + Project + Cumulative
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.3	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	195	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	243	pc/h
Highest directional split proportion (note-2)	139	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	2.2	mi/h
Average travel speed, ATS	50.8	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	225	pc/h
Highest directional split proportion (note-2)	128	
Base percent time-spent-following, BPTSF	17.9	%
Adj.for directional distribution and no-passing zones, fd/np	19.5	
Percent time-spent-following, PTSF	37.4	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.08	
Peak 15-min vehicle-miles of travel, VMT15	626	veh-mi
Peak-hour vehicle-miles of travel, VMT60	2204	veh-mi
Peak 15-min total travel time, TT15	12.3	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 9/12/06
Analysis Time Period PM
Highway SR-94
From/To Sheridan Rd to White Star
Jurisdiction Caltrans
Analysis Year Existing + Project + Cumulative
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.3	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	165	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	206	pc/h
Highest directional split proportion (note-2)	117	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	1.9	mi/h
Average travel speed, ATS	51.3	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	190	pc/h
Highest directional split proportion (note-2)	108	
Base percent time-spent-following, BPTSF	15.4	%
Adj.for directional distribution and no-passing zones, fd/np	19.5	
Percent time-spent-following, PTSF	34.9	%

Level of Service and Other Performance Measures

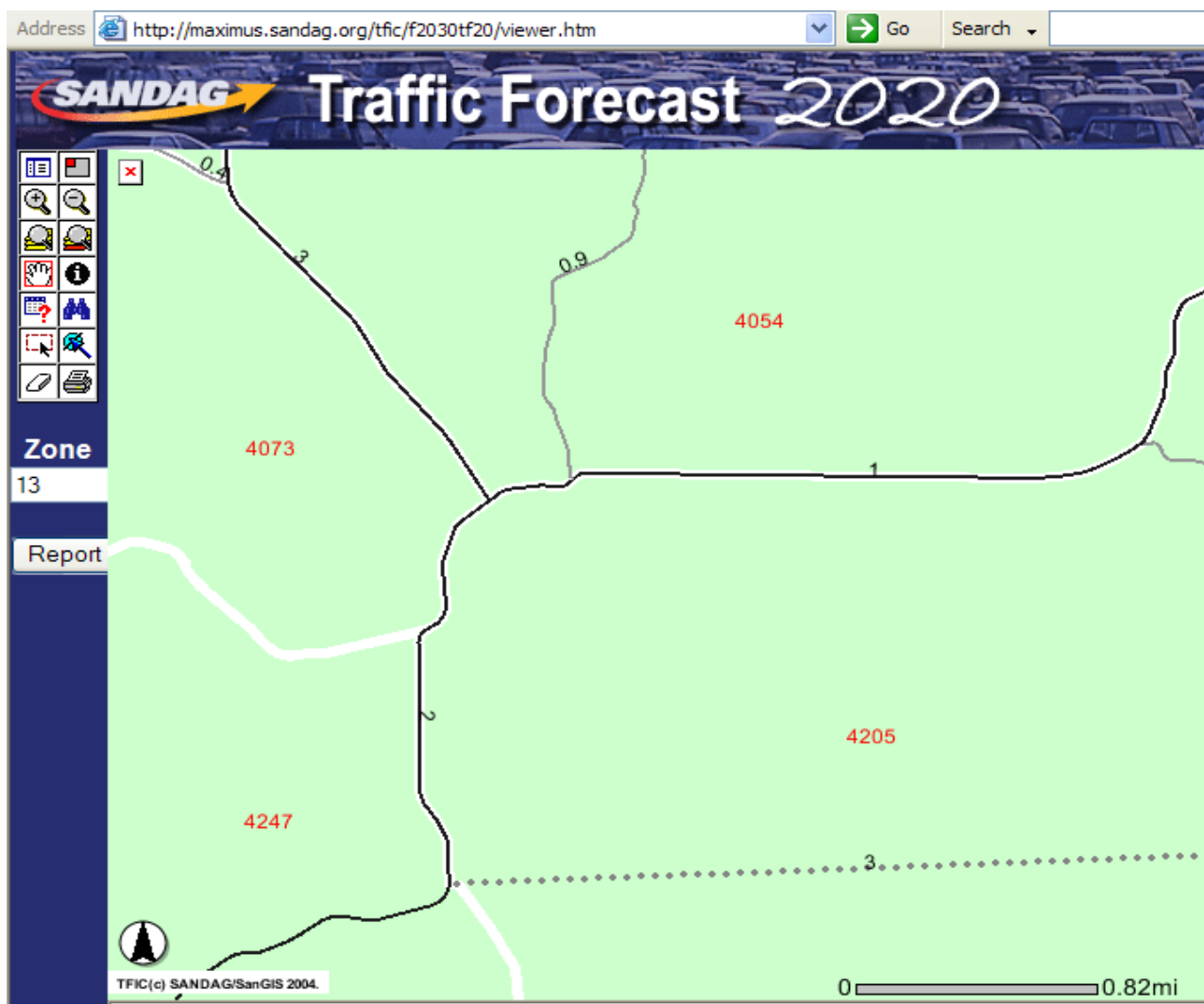
Level of service, LOS	B	
Volume to capacity ratio, v/c	0.06	
Peak 15-min vehicle-miles of travel, VMT15	530	veh-mi
Peak-hour vehicle-miles of travel, VMT60	1865	veh-mi
Peak 15-min total travel time, TT15	10.3	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

APPENDIX M

YEAR 2020 SANDAG ADTS, GROWTH FACTORS, AND FORECASTED ADTS



ADT GROWTH CALCULATIONS

SR-94 south of Buchman Springs Road

Year	Delta in Years	ADT		Change in ADT	% change for year shown	Average Growth Factor Per Year
1993		1,500	Caltrans Historical Data			
1995	2	2,200		700	15.9%	
1997	2	1,800		-400	-11.1%	
1998	1	1,750		-50	-2.9%	
1999	1	1,750		0	0.0%	
2000	1	1,800		50	2.8%	
2001	1	2,000		200	10.0%	
2002	1	2,000		0	0.0%	
2003	4	2,150		150	1.7%	3.33%

SR-94 east of Buchman Springs Road

Year	Delta in Years	ADT		Change in ADT	% change for year shown	Average Growth Factor Per Year
1993		1,600	Caltrans Historical Data			
1995	2	1,850		250	6.8%	
1997	2	1,550		-300	-9.7%	
1998	1	1,500		-50	-3.3%	
1999	1	1,500		0	0.0%	
2000	1	1,650		150	9.1%	
2001	1	1,800		150	8.3%	
2002	1	1,750		-50	-2.9%	
2003	4	1,750		0	0.0%	0.72%

For Buckman Springs Road and Sheridan Road, the average of the above growth factors was used, which equals 2.03%

















Street Segment	SR-94	SR-94	SR-94	SR-94	Buckman Spr	Sheridan Rd
From Tecate Rd	Forest Gate	Buckman Spr	Sheridan Rd	I-8	SR-94	SR-94
To Forest Gate	Buckman Spr	Sheridan Rd	White Star	SR-94	Jeb Stuart Rd	
Existing+Cumulative ADT	2,610	2,085	4,164	2,330	4,922	818
Growth Factor (1)	3.33%	3.33%	0.72%	0.72%	2.03%	2.03%
2020 (based on E+C)	4,088	3,265	4,674	2,615	6,621	1,100











(1) For Buckman Springs Road and Sheridan Road, the average growth factors was used, which equals 2.03% while the growth factors for SR-94 were based on the actual growth factor calculated on the previous page.










																BUILD-OUT															
Time	DATE	TIME	INTID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR				
1) SR-94/Forest Gate Road					2610			2085			1000			1000			4088				3265				1100				1300		
AM E	4/21/2004	700	1	2	41	14	47	33	2	2	0	1	4	1	32																
AM 2020	4/25/2004	700		10	60	30	70	50	10	10	10	10	20	10	40	0.001	0.016	0.005	0.023	0.016	0.001	0.002	0.000	0.001	0.004	0.001	0.032				
PM E	4/21/2004	1700		3	29	1	35	37	8	11	8	1	13	11	43																
PM 2020	4/25/2004	1700		10	50	10	50	60	10	20	10	10	30	20	60	0.001	0.011	0.000	0.017	0.018	0.004	0.011	0.008	0.001	0.013	0.011	0.043				
2) SR-94/Sheridan Road					818			0			4164			2330			1100				0				4674				2615		
AM E	4/21/2004	700	2	10	0	6	0	0	0	0	50	22	16	42	0																
AM 2020	4/25/2004	700		20	10	20	#####	#####	#####	10	60	30	30	60	10	0.012	0.000	0.007	#####	#DIV/0!	#####	0.000	0.012	0.005	0.007	0.018	0.000				
PM E	4/21/2004	1700		21	0	12	0	0	0	0	63	8	9	44	0																
PM 2020	4/25/2004	1700		40	10	20	#####	#####	#####	10	80	20	20	60	10	0.026	0.000	0.015	#####	#DIV/0!	#####	0.000	0.015	0.002	0.004	0.019	0.000				
3) Sheridan Rd/Project Driveway					818			818			0			764			1100				1100				0				764		
AM E	4/21/2004	700	3	0	16	0	0	20	0	0	0	0	0	0	0																
AM 2020	4/25/2004	700		0	30	0	0	50	0	#####	#####	#####	0	0	0	0.000	0.020	0.000	0.000	0.024	0.000	#####	#####	#####	0.000	0.000	0.000				
PM E	4/21/2004	1700		0	15	0	0	21	0	0	0	0	0	0	0																
PM 2020	4/25/2004	1700		0	30	0	0	40	0	#####	#####	#####	0	0	0	0.000	0.018	0.000	0.000	0.026	0.000	#####	#####	#####	0.000	0.000	0.000				

















APPENDIX N










YEAR 2020 WITHOUT AND WITH PROJECT INTERSECTION LEVEL OF SERVICE CALCUALTIONS










												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	10	10	10	20	10	40	10	60	30	70	50	10
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	11	11	11	21	11	42	11	63	32	74	53	11
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	353	321	58	321	311	79	63			95		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	353	321	58	321	311	79	63			95		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	98	98	99	96	98	96	99			95		
cM capacity (veh/h)	544	563	1008	590	571	982	1539			1499		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	32	74	105	137								
Volume Left	11	21	11	74								
Volume Right	11	42	32	11								
cSH	651	759	1539	1499								
Volume to Capacity	0.05	0.10	0.01	0.05								
Queue Length 95th (ft)	4	8	1	4								
Control Delay (s)	10.8	10.2	0.8	4.2								
Lane LOS	B	B	A	A								
Approach Delay (s)	10.8	10.2	0.8	4.2								
Approach LOS	B	B										
Intersection Summary												
Average Delay			5.1									
Intersection Capacity Utilization			25.6%			ICU Level of Service				A		
Analysis Period (min)			15									

















						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Volume (veh/h)	60	30	30	60	20	20
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	63	32	32	63	21	21
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type					None	
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			95		205	79
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			95		205	79
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			98		97	98
cM capacity (veh/h)			1499		767	982
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	95	95	42			
Volume Left	0	32	21			
Volume Right	32	0	21			
cSH	1700	1499	861			
Volume to Capacity	0.06	0.02	0.05			
Queue Length 95th (ft)	0	2	4			
Control Delay (s)	0.0	2.6	9.4			
Lane LOS		A	A			
Approach Delay (s)	0.0	2.6	9.4			
Approach LOS			A			
Intersection Summary						
Average Delay			2.8			
Intersection Capacity Utilization		21.5%		ICU Level of Service		A
Analysis Period (min)			15			











						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Volume (veh/h)	1	1	30	1	2	50
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	1	1	32	1	2	53
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	89	32			33	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	89	32			33	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	100			100	
cM capacity (veh/h)	910	1042			1579	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	2	33	55			
Volume Left	1	0	2			
Volume Right	1	1	0			
cSH	972	1700	1579			
Volume to Capacity	0.00	0.02	0.00			
Queue Length 95th (ft)	0	0	0			
Control Delay (s)	8.7	0.0	0.3			
Lane LOS	A		A			
Approach Delay (s)	8.7	0.0	0.3			
Approach LOS	A					
Intersection Summary						
Average Delay		0.4				
Intersection Capacity Utilization		14.3%		ICU Level of Service		A
Analysis Period (min)		15				










												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	20	10	10	30	20	60	10	50	10	50	60	10
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	21	11	11	32	21	63	11	53	11	53	63	11
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	326	258	68	268	258	58	74			63		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	326	258	68	268	258	58	74			63		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	96	98	99	95	97	94	99			97		
cM capacity (veh/h)	555	620	995	647	620	1008	1526			1539		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	42	116	74	126								
Volume Left	21	32	11	53								
Volume Right	11	63	11	11								
cSH	643	796	1526	1539								
Volume to Capacity	0.07	0.15	0.01	0.03								
Queue Length 95th (ft)	5	13	1	3								
Control Delay (s)	11.0	10.3	1.1	3.2								
Lane LOS	B	B	A	A								
Approach Delay (s)	11.0	10.3	1.1	3.2								
Approach LOS	B	B										
Intersection Summary												
Average Delay			6.0									
Intersection Capacity Utilization			26.8%	ICU Level of Service						A		
Analysis Period (min)			15									

















						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Volume (veh/h)	80	20	20	60	40	20
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	84	21	21	63	42	21
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			105		200	95
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			105		200	95
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		95	98
cM capacity (veh/h)			1486		777	962
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	105	84	63			
Volume Left	0	21	42			
Volume Right	21	0	21			
cSH	1700	1486	831			
Volume to Capacity	0.06	0.01	0.08			
Queue Length 95th (ft)	0	1	6			
Control Delay (s)	0.0	1.9	9.7			
Lane LOS		A	A			
Approach Delay (s)	0.0	1.9	9.7			
Approach LOS			A			
Intersection Summary						
Average Delay		3.1				
Intersection Capacity Utilization		21.4%		ICU Level of Service		A
Analysis Period (min)		15				










						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Volume (veh/h)	2	2	30	2	2	40
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	2	2	32	2	2	42
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	79	33			34	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	79	33			34	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	100			100	
cM capacity (veh/h)	922	1041			1578	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	4	34	44			
Volume Left	2	0	2			
Volume Right	2	2	0			
cSH	978	1700	1578			
Volume to Capacity	0.00	0.02	0.00			
Queue Length 95th (ft)	0	0	0			
Control Delay (s)	8.7	0.0	0.4			
Lane LOS	A		A			
Approach Delay (s)	8.7	0.0	0.4			
Approach LOS	A					
Intersection Summary						
Average Delay		0.6				
Intersection Capacity Utilization		13.7%		ICU Level of Service		A
Analysis Period (min)		15				










												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	10	10	10	37	10	40	10	60	57	70	50	10
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	11	11	11	39	11	42	11	63	60	74	53	11
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	367	349	58	335	325	93	63			123		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	367	349	58	335	325	93	63			123		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	98	98	99	93	98	96	99			95		
cM capacity (veh/h)	532	542	1008	577	559	964	1539			1464		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	32	92	134	137								
Volume Left	11	39	11	74								
Volume Right	11	42	60	11								
cSH	636	704	1539	1464								
Volume to Capacity	0.05	0.13	0.01	0.05								
Queue Length 95th (ft)	4	11	1	4								
Control Delay (s)	11.0	10.9	0.6	4.3								
Lane LOS	B	B	A	A								
Approach Delay (s)	11.0	10.9	0.6	4.3								
Approach LOS	B	B										
Intersection Summary												
Average Delay			5.1									
Intersection Capacity Utilization			27.7%	ICU Level of Service						A		
Analysis Period (min)			15									

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Volume (veh/h)	60	57	43	60	37	29
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	63	60	45	63	39	31
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			123		247	93
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			123		247	93
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			97		95	97
cM capacity (veh/h)			1464		719	964
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	123	108	69			
Volume Left	0	45	39			
Volume Right	60	0	31			
cSH	1700	1464	809			
Volume to Capacity	0.07	0.03	0.09			
Queue Length 95th (ft)	0	2	7			
Control Delay (s)	0.0	3.3	9.9			
Lane LOS		A	A			
Approach Delay (s)	0.0	3.3	9.9			
Approach LOS			A			
Intersection Summary						
Average Delay			3.5			
Intersection Capacity Utilization		23.1%		ICU Level of Service		A
Analysis Period (min)		15				

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Volume (veh/h)	39	47	30	60	73	50
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	41	49	32	63	77	53
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	269	63			95	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	269	63			95	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	94	95			95	
cM capacity (veh/h)	683	1001			1499	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	91	95	129			
Volume Left	41	0	77			
Volume Right	49	63	0			
cSH	827	1700	1499			
Volume to Capacity	0.11	0.06	0.05			
Queue Length 95th (ft)	9	0	4			
Control Delay (s)	9.9	0.0	4.6			
Lane LOS	A		A			
Approach Delay (s)	9.9	0.0	4.6			
Approach LOS	A					
Intersection Summary						
Average Delay		4.7				
Intersection Capacity Utilization		25.6%		ICU Level of Service		A
Analysis Period (min)		15				

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Volume (veh/h)	20	10	10	36	20	60	10	50	15	50	60	10
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	21	11	11	38	21	63	11	53	16	53	63	11
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	329	263	68	271	261	61	74			68		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	329	263	68	271	261	61	74			68		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	96	98	99	94	97	94	99			97		
cM capacity (veh/h)	552	616	995	645	618	1005	1526			1533		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	42	122	79	126								
Volume Left	21	38	11	53								
Volume Right	11	63	16	11								
cSH	640	784	1526	1533								
Volume to Capacity	0.07	0.16	0.01	0.03								
Queue Length 95th (ft)	5	14	1	3								
Control Delay (s)	11.0	10.4	1.0	3.3								
Lane LOS	B	B	A	A								
Approach Delay (s)	11.0	10.4	1.0	3.3								
Approach LOS	B	B										
Intersection Summary												
Average Delay			6.0									
Intersection Capacity Utilization			27.3%		ICU Level of Service					A		
Analysis Period (min)			15									

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Volume (veh/h)	80	25	22	60	46	23
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	84	26	23	63	48	24
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type					None	
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			111		207	97
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			111		207	97
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			98		94	97
cM capacity (veh/h)			1479		769	959
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	111	86	73			
Volume Left	0	23	48			
Volume Right	26	0	24			
cSH	1700	1479	824			
Volume to Capacity	0.07	0.02	0.09			
Queue Length 95th (ft)	0	1	7			
Control Delay (s)	0.0	2.1	9.8			
Lane LOS		A	A			
Approach Delay (s)	0.0	2.1	9.8			
Approach LOS			A			
Intersection Summary						
Average Delay			3.3			
Intersection Capacity Utilization		22.1%		ICU Level of Service	A	
Analysis Period (min)			15			

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Volume (veh/h)	14	17	30	10	13	40
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	15	18	32	11	14	42
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	106	37			42	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	106	37			42	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	98	98			99	
cM capacity (veh/h)	884	1035			1567	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	33	42	56			
Volume Left	15	0	14			
Volume Right	18	11	0			
cSH	961	1700	1567			
Volume to Capacity	0.03	0.02	0.01			
Queue Length 95th (ft)	3	0	1			
Control Delay (s)	8.9	0.0	1.8			
Lane LOS	A		A			
Approach Delay (s)	8.9	0.0	1.8			
Approach LOS	A					
Intersection Summary						
Average Delay		3.0				
Intersection Capacity Utilization		19.5%		ICU Level of Service		A
Analysis Period (min)		15				

APPENDIX O

YEAR 2020 WITHOUT AND WITH PROJECT TWO-LANE HIGHWAY (HCM) CALCUALTIONS

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period AM
Highway SR-94
From/To Tecate Rd to Forest Gate Rd
Jurisdiction Caltrans
Analysis Year 2020
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.8	mi	% Recreational vehicles	4	%
Terrain type	Rolling		% No-passing zones	100	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	226	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	0.71	
PCE for trucks, ET	2.5	
PCE for RVs, ER	1.1	
Heavy-vehicle adjustment factor,	0.824	
Two-way flow rate, (note-1) vp	439	pc/h
Highest directional split proportion (note-2)	294	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	4.4	mi/h
Average travel speed, ATS	47.0	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	0.77	
PCE for trucks, ET	1.8	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.899	
Two-way flow rate,(note-1) vp	371	pc/h
Highest directional split proportion (note-2)	249	
Base percent time-spent-following, BPTSF	27.8	%
Adj.for directional distribution and no-passing zones, fd/np	23.2	
Percent time-spent-following, PTSF	51.0	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.14	
Peak 15-min vehicle-miles of travel, VMT15	758	veh-mi
Peak-hour vehicle-miles of travel, VMT60	2667	veh-mi
Peak 15-min total travel time, TT15	16.1	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period PM
Highway SR-94
From/To Tecate Rd to Forest Gate Rd
Jurisdiction Caltrans
Analysis Year 2020
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.8	mi	% Recreational vehicles	4	%
Terrain type	Rolling		% No-passing zones	100	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	280	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	0.71	
PCE for trucks, ET	2.5	
PCE for RVs, ER	1.1	
Heavy-vehicle adjustment factor,	0.824	
Two-way flow rate, (note-1) vp	544	pc/h
Highest directional split proportion (note-2)	364	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	4.1	mi/h
Average travel speed, ATS	46.5	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	0.77	
PCE for trucks, ET	1.8	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.899	
Two-way flow rate,(note-1) vp	460	pc/h
Highest directional split proportion (note-2)	308	
Base percent time-spent-following, BPTSF	33.3	%
Adj.for directional distribution and no-passing zones, fd/np	22.3	
Percent time-spent-following, PTSF	55.5	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.17	
Peak 15-min vehicle-miles of travel, VMT15	939	veh-mi
Peak-hour vehicle-miles of travel, VMT60	3304	veh-mi
Peak 15-min total travel time, TT15	20.2	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period AM
Highway SR-94
From/To Forest Gate Rd to Buckman Spr
Jurisdiction Caltrans
Analysis Year 2020
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	1.4	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	181	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	226	pc/h
Highest directional split proportion (note-2)	151	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	2.1	mi/h
Average travel speed, ATS	51.0	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	209	pc/h
Highest directional split proportion (note-2)	140	
Base percent time-spent-following, BPTSF	16.8	%
Adj.for directional distribution and no-passing zones, fd/np	21.2	
Percent time-spent-following, PTSF	38.0	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.07	
Peak 15-min vehicle-miles of travel, VMT15	72	veh-mi
Peak-hour vehicle-miles of travel, VMT60	253	veh-mi
Peak 15-min total travel time, TT15	1.4	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period PM
Highway SR-94
From/To Forest Gate Rd to Buckman Spr
Jurisdiction Caltrans
Analysis Year 2020
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	1.4	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	223	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	278	pc/h
Highest directional split proportion (note-2)	186	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	2.4	mi/h
Average travel speed, ATS	50.3	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	257	pc/h
Highest directional split proportion (note-2)	172	
Base percent time-spent-following, BPTSF	20.2	%
Adj.for directional distribution and no-passing zones, fd/np	20.7	
Percent time-spent-following, PTSF	41.0	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.09	
Peak 15-min vehicle-miles of travel, VMT15	89	veh-mi
Peak-hour vehicle-miles of travel, VMT60	312	veh-mi
Peak 15-min total travel time, TT15	1.8	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

Two-Way Two-Lane Highway Segment Analysis

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period AM
Highway SR-94
From/To Buckman Springs to Sheridan Rd
Jurisdiction Caltrans
Analysis Year 2020
Description St. Adelaide

Input Data

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	0.8	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	335	veh/h
Directional split	57 / 43	%

Average Travel Speed

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	418	pc/h
Highest directional split proportion (note-2)	238	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	3.1	mi/h
Average travel speed, ATS	48.5	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	386	pc/h
Highest directional split proportion (note-2)	220	
Base percent time-spent-following, BPTSF	28.8	%
Adj.for directional distribution and no-passing zones, fd/np	19.2	
Percent time-spent-following, PTSF	48.0	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.13	
Peak 15-min vehicle-miles of travel, VMT15	76	veh-mi
Peak-hour vehicle-miles of travel, VMT60	268	veh-mi
Peak 15-min total travel time, TT15	1.6	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period PM
Highway SR-94
From/To Buckman Springs to Sheridan Rd
Jurisdiction Caltrans
Analysis Year 2020
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	0.8	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	363	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	453	pc/h
Highest directional split proportion (note-2)	258	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	3.0	mi/h
Average travel speed, ATS	48.3	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	418	pc/h
Highest directional split proportion (note-2)	238	
Base percent time-spent-following, BPTSF	30.7	%
Adj.for directional distribution and no-passing zones, fd/np	19.0	
Percent time-spent-following, PTSF	49.7	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.14	
Peak 15-min vehicle-miles of travel, VMT15	83	veh-mi
Peak-hour vehicle-miles of travel, VMT60	290	veh-mi
Peak 15-min total travel time, TT15	1.7	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
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-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period AM
Highway SR-94
From/To Sheridan Rd to White Star
Jurisdiction Caltrans
Analysis Year 2020
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.3	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	175	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	218	pc/h
Highest directional split proportion (note-2)	124	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	2.0	mi/h
Average travel speed, ATS	51.1	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	202	pc/h
Highest directional split proportion (note-2)	115	
Base percent time-spent-following, BPTSF	16.3	%
Adj.for directional distribution and no-passing zones, fd/np	19.5	
Percent time-spent-following, PTSF	35.8	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.07	
Peak 15-min vehicle-miles of travel, VMT15	562	veh-mi
Peak-hour vehicle-miles of travel, VMT60	1978	veh-mi
Peak 15-min total travel time, TT15	11.0	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
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-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period PM
Highway SR-94
From/To Sheridan Rd to White Star
Jurisdiction Caltrans
Analysis Year 2020
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.3	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	160	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	200	pc/h
Highest directional split proportion (note-2)	114	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	1.9	mi/h
Average travel speed, ATS	51.3	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	184	pc/h
Highest directional split proportion (note-2)	105	
Base percent time-spent-following, BPTSF	14.9	%
Adj.for directional distribution and no-passing zones, fd/np	19.5	
Percent time-spent-following, PTSF	34.5	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.06	
Peak 15-min vehicle-miles of travel, VMT15	514	veh-mi
Peak-hour vehicle-miles of travel, VMT60	1808	veh-mi
Peak 15-min total travel time, TT15	10.0	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period AM
Highway SR-94
From/To Tecate Rd to Forest Gate Rd
Jurisdiction Caltrans
Analysis Year 2020 + Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.8	mi	% Recreational vehicles	4	%
Terrain type	Rolling		% No-passing zones	100	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	270	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	0.71	
PCE for trucks, ET	2.5	
PCE for RVs, ER	1.1	
Heavy-vehicle adjustment factor,	0.824	
Two-way flow rate, (note-1) vp	525	pc/h
Highest directional split proportion (note-2)	352	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	4.1	mi/h
Average travel speed, ATS	46.6	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	0.77	
PCE for trucks, ET	1.8	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.899	
Two-way flow rate,(note-1) vp	443	pc/h
Highest directional split proportion (note-2)	297	
Base percent time-spent-following, BPTSF	32.3	%
Adj.for directional distribution and no-passing zones, fd/np	22.5	
Percent time-spent-following, PTSF	54.7	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.16	
Peak 15-min vehicle-miles of travel, VMT15	905	veh-mi
Peak-hour vehicle-miles of travel, VMT60	3186	veh-mi
Peak 15-min total travel time, TT15	19.4	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period PM
Highway SR-94
From/To Tecate Rd to Forest Gate Rd
Jurisdiction Caltrans
Analysis Year 2020 + Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.8	mi	% Recreational vehicles	4	%
Terrain type	Rolling		% No-passing zones	100	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	291	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	0.71	
PCE for trucks, ET	2.5	
PCE for RVs, ER	1.1	
Heavy-vehicle adjustment factor,	0.824	
Two-way flow rate, (note-1) vp	565	pc/h
Highest directional split proportion (note-2)	379	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	4.0	mi/h
Average travel speed, ATS	46.4	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	0.77	
PCE for trucks, ET	1.8	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.899	
Two-way flow rate,(note-1) vp	478	pc/h
Highest directional split proportion (note-2)	320	
Base percent time-spent-following, BPTSF	34.3	%
Adj.for directional distribution and no-passing zones, fd/np	22.1	
Percent time-spent-following, PTSF	56.4	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.18	
Peak 15-min vehicle-miles of travel, VMT15	976	veh-mi
Peak-hour vehicle-miles of travel, VMT60	3434	veh-mi
Peak 15-min total travel time, TT15	21.0	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period AM
Highway SR-94
From/To Forest Gate Rd to Buckman Spr
Jurisdiction Caltrans
Analysis Year 2020 + Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	1.4	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	181	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	226	pc/h
Highest directional split proportion (note-2)	151	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	2.1	mi/h
Average travel speed, ATS	51.0	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	209	pc/h
Highest directional split proportion (note-2)	140	
Base percent time-spent-following, BPTSF	16.8	%
Adj.for directional distribution and no-passing zones, fd/np	21.2	
Percent time-spent-following, PTSF	38.0	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.07	
Peak 15-min vehicle-miles of travel, VMT15	72	veh-mi
Peak-hour vehicle-miles of travel, VMT60	253	veh-mi
Peak 15-min total travel time, TT15	1.4	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period PM
Highway SR-94
From/To Forest Gate Rd to Buckman Spr
Jurisdiction Caltrans
Analysis Year 2020 + Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	1.4	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	223	veh/h
Directional split	67 / 33	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	278	pc/h
Highest directional split proportion (note-2)	186	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	2.4	mi/h
Average travel speed, ATS	50.3	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	257	pc/h
Highest directional split proportion (note-2)	172	
Base percent time-spent-following, BPTSF	20.2	%
Adj.for directional distribution and no-passing zones, fd/np	20.7	
Percent time-spent-following, PTSF	41.0	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.09	
Peak 15-min vehicle-miles of travel, VMT15	89	veh-mi
Peak-hour vehicle-miles of travel, VMT60	312	veh-mi
Peak 15-min total travel time, TT15	1.8	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period AM
Highway SR-94
From/To Buckman Springs to Sheridan Rd
Jurisdiction Caltrans
Analysis Year 2020 + Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	0.8	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	379	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	473	pc/h
Highest directional split proportion (note-2)	270	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	3.0	mi/h
Average travel speed, ATS	48.2	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	437	pc/h
Highest directional split proportion (note-2)	249	
Base percent time-spent-following, BPTSF	31.9	%
Adj.for directional distribution and no-passing zones, fd/np	18.8	
Percent time-spent-following, PTSF	50.7	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.15	
Peak 15-min vehicle-miles of travel, VMT15	86	veh-mi
Peak-hour vehicle-miles of travel, VMT60	303	veh-mi
Peak 15-min total travel time, TT15	1.8	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period PM
Highway SR-94
From/To Buckman Springs to Sheridan Rd
Jurisdiction Caltrans
Analysis Year 2020 + Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	0.8	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	374	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	467	pc/h
Highest directional split proportion (note-2)	266	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	3.0	mi/h
Average travel speed, ATS	48.2	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	431	pc/h
Highest directional split proportion (note-2)	246	
Base percent time-spent-following, BPTSF	31.5	%
Adj.for directional distribution and no-passing zones, fd/np	18.9	
Percent time-spent-following, PTSF	50.4	%

Level of Service and Other Performance Measures

Level of service, LOS	C	
Volume to capacity ratio, v/c	0.15	
Peak 15-min vehicle-miles of travel, VMT15	85	veh-mi
Peak-hour vehicle-miles of travel, VMT60	299	veh-mi
Peak 15-min total travel time, TT15	1.8	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period AM
Highway SR-94
From/To Sheridan Rd to White Star
Jurisdiction Caltrans
Analysis Year 2020 + Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.3	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	197	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	246	pc/h
Highest directional split proportion (note-2)	140	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	2.2	mi/h
Average travel speed, ATS	50.7	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	227	pc/h
Highest directional split proportion (note-2)	129	
Base percent time-spent-following, BPTSF	18.1	%
Adj.for directional distribution and no-passing zones, fd/np	19.5	
Percent time-spent-following, PTSF	37.5	%

Level of Service and Other Performance Measures

Level of service, LOS	B	
Volume to capacity ratio, v/c	0.08	
Peak 15-min vehicle-miles of travel, VMT15	632	veh-mi
Peak-hour vehicle-miles of travel, VMT60	2226	veh-mi
Peak 15-min total travel time, TT15	12.5	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

Phone: Fax:
E-Mail:

-----Two-Way Two-Lane Highway Segment Analysis-----

Analyst JR
Agency/Co. County
Date Performed 4/5/2005
Analysis Time Period PM
Highway SR-94
From/To Sheridan Rd to White Star
Jurisdiction Caltrans
Analysis Year 2020 + Project
Description St. Adelaide

-----Input Data-----

Highway class	Class 1				
Shoulder width	1.0	ft	Peak-hour factor, PHF	0.88	
Lane width	12.0	ft	% Trucks and buses	14	%
Segment length	11.3	mi	% Recreational vehicles	4	%
Terrain type	Level		% No-passing zones	50	%
Grade: Length		mi	Access points/mi	4	/mi
Up/down		%			

Two-way hourly volume, V	165	veh/h
Directional split	57 / 43	%

-----Average Travel Speed-----

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.7	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor,	0.911	
Two-way flow rate, (note-1) vp	206	pc/h
Highest directional split proportion (note-2)	117	pc/h
Free-Flow Speed from Field Measurement:		
Field measured speed, SFM	-	mi/h
Observed volume, Vf	-	veh/h
Estimated Free-Flow Speed:		
Base free-flow speed, BFFS	60.0	mi/h
Adj. for lane and shoulder width, fLS	4.2	mi/h
Adj. for access points, fA	1.0	mi/h
Free-flow speed, FFS	54.8	mi/h
Adjustment for no-passing zones, fnp	1.9	mi/h
Average travel speed, ATS	51.3	mi/h

Percent Time-Spent-Following

Grade adjustment factor, fG	1.00	
PCE for trucks, ET	1.1	
PCE for RVs, ER	1.0	
Heavy-vehicle adjustment factor, fHV	0.986	
Two-way flow rate,(note-1) vp	190	pc/h
Highest directional split proportion (note-2)	108	
Base percent time-spent-following, BPTSF	15.4	%
Adj.for directional distribution and no-passing zones, fd/np	19.5	
Percent time-spent-following, PTSF	34.9	%

Level of Service and Other Performance Measures

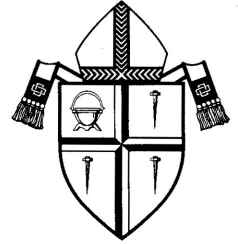
Level of service, LOS	B	
Volume to capacity ratio, v/c	0.06	
Peak 15-min vehicle-miles of travel, VMT15	530	veh-mi
Peak-hour vehicle-miles of travel, VMT60	1865	veh-mi
Peak 15-min total travel time, TT15	10.3	veh-h

Notes:

1. If vp >= 3200 pc/h, terminate analysis-the LOS is F.
2. If highest directional split vp >= 1700 pc/h, terminate analysis-the LOS is F.

APPENDIX P

PROJECT APPLICANT TIF AGREEMENT LETTER



Construction Services

May 5, 2006

Mr. Gary Pryor, Director
County of San Diego
Department of Planning and Land Use
5201 Ruffin Road, Suite B
San Diego, CA 92123

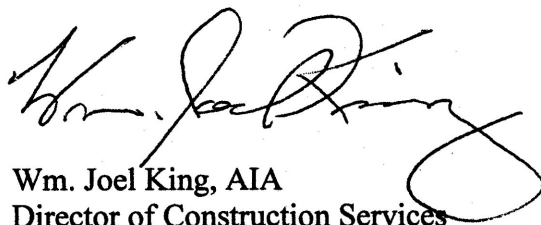
Dear Mr. Pryor

The applicant and owner for the Saint Adelaide Catholic Church in Campo (MUP 04-056) agrees to participate in the San Diego County TIF (traffic impact fee) program to mitigate all potential cumulative traffic impacts associated with this project. We understand that the TIF fees for the church buildings are based upon the square footage of the church building and the TIF fee for K-8 schools is at a rate per student. These fees are payable at the time building permits are issued and are phased with the project.

TIF fees will be based upon the TIF fee schedule in effect at the time the building permits are issued. The current rates for church construction are \$1,834.00 per one-thousand (1000) square feet and the rate for K-8 schools is \$330.00 per student.

The applicant understands that these fees are subject to change as the TIF is updated annually and the fees are adjusted to reflect the engineering cost index.

Sincerely,



Wm. Joel King, AIA
Director of Construction Services